

Engineering Design File

Transportation of the PM-2A Tanks from the TSF-26 Site to the TAN-607A High Bay

Portage Project No.: 2703.00
Project Title: PM-2A Remediation Phase I



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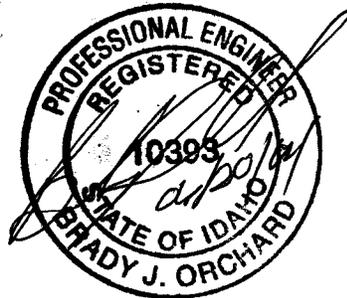
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3. Subtask: Tank Excavation, Transport, and Storage
4. Title: Transport of the PM-2A Tanks from the TSF-26 Site to the TAN-607A High Bay

5. Summary:
This engineering design file evaluates the impact of loads imposed during the transportation of the PM-2A tanks to the Test Area North-607A High Bay may have on underground utilities within the transport path. This engineering design file also identifies overhead power and communication lines in the transport path that require rerouting or lifting and fill/grading requirements for the transport path.

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I. INTRODUCTION AND PURPOSE

The purpose of this engineering design file (EDF) is to evaluate the impact of loads imposed on underground utilities located beneath the transport path for the PM-2A tanks from the Technical Support Facility (TSF)-26 site to the Test Area North (TAN)-607A High Bay and to determine the need for modification of the transport path.

This EDF also addresses necessary relocation of overhead communication and electrical lines and transport path preparation activities (i.e., fill and grade) necessary for transport of the PM-2A tanks from the TSF-26 site to the TAN-607A High Bay.

2. UNDERGROUND UTILITIES

A variety of underground piping is located beneath the proposed transport path. Table 1 provides a summary of the buried piping between the TSF-26 site and the TAN-607A High Bay. This piping was evaluated to determine whether the load imposed during transport of the PM-2A tanks would detrimentally impact the buried piping.

Table 1. Summary of the buried piping between the TSF-26 site and the TAN-607A High Bay.

Pipe Number	Description/Type ^a	Size	Material of Construction	Depth Below Grade at Crossing	Status
10" SSD-10001	Sanitary sewer from TSF area to TAN-711	10 in.	Concrete	8.21 ft	In service
Unidentified line (see Drawing 423185, Sheet 3 of 6)	Fire water west of Firehole Road	8 in.	Steel	Approx. 5 ft	Abandoned
10" SWD-10035	Service waste to TAN injection well	10 in.	Concrete	5 ft	Not in service
Unidentified line (see Drawing 423185, Sheet 3 of 6)	Fire water main west of Firehole Road	4 in.	Steel	Approx. 5 ft	In service
Unidentified line (see Drawing 217515)	Water line east of Firehole Road	4 in. (assumed)	Steel	Approx. 5 ft	In service
Unidentified line (see Drawing 217515)	Water line from TAN-609	4 in. (assumed)	Steel	Approx. 5 ft	Abandoned
Unidentified line (see Drawing 423185, Sheet 3 of 6)	Fire water line from TAN-609	4 in. (assumed)	Steel	Approx. 5 ft	Abandoned
Unidentified line (see Drawing 423185, Sheet 3 of 6)	Fire water main	4 in. (assumed)	Steel	Approx. 5 ft	In service
8" SSD-10009	Sanitary sewer from TAN-609	8 in.	Concrete	Approx. 7 ft	Abandoned

Table 1. (continued).

Pipe Number	Description/Type ^a	Size	Material of Construction	Depth Below Grade at Crossing	Status
8" SSD-10002	Sanitary sewer MH#1 to MH#2	8 in.	Concrete	Approx. 10 ft	In service
8" SWD-10036	Service waste from TAN-607	8 in.	Concrete	Approx. 5 ft	Abandoned
8" SWD-10038	Service waste from TAN-609	8 in.	Concrete	Approx. 5 ft	Abandoned
Unidentified line (see Drawing 217515)	Water across Snake Avenue	8 in.	Steel	Approx. 5 ft	In service
Unidentified line (see Drawing 217515)	Water west of TAN-607	8 in.	Steel encased in concrete under tracks	Approx. 5 ft	In service
10" SSD-10001	Sanitary sewer	10 in.	Concrete encased in concrete under tracks	Approx. 9 ft	In service

a. Buried Waste Line Register for NRTS (Paige 1972).

Based on a review of the piping, the two concrete sanitary sewer lines were determined to have the highest potential for damage resulting from transport of the PM-2A tanks. The 10-in. line is approximately 8 ft below the ramp from the TSF-26 site. The 8-in. line is approximately 10 ft below Snake Avenue.

2.1 Imposed Load

The weight of the transporter, tank, and saddle supports is estimated to be 237,000 lb. The wheel load is estimated to be 4,032 lb/ft² (Duratek Drawing C-067-RP0003-003).

2.2 Breaking Strength of the 10-in. Sewer Pipe

Available records indicate that the sanitary sewer line is 10-in. concrete pipe (R. M. Parsons Drawing 1229-5-ANP/GE-3-607-U106). No additional design data have been identified for the sanitary sewer line. Therefore, it is assumed that line is bedded properly and the crushing strength is equal to 2,000 lb/lf. (*Design Data 25*; see Attachment 1 [ACPA 1974])

2.3 Load Analysis

The vertical external load on the 10-in. sanitary sewer line is calculated with the following assumptions:

- The soil are relatively dry soils conservatively estimated to weigh, $W = 120 \text{ lb/ft}^3$
- The height of the soil above the top of the pipe, H , is 8.21 ft.

2.3.1 Live Load

- Impact factor (1.75, most conservative value)
- Percent of wheel load on pipe (from table on Page 40 of Clay Pipe Engineering Manual; see Attachment 1 [NCPI 1968]) = 0.60%
- From table on Page 101 of the Concrete Pipe Design Manual (see Attachment 1 [ACPA 1970]), a 10-in. pipe, the weight of the soil at 120 lb/ft^3 , and a 8-ft depth the maximum worst-case load = 1,193 lb
- The wheel load transmitted to the pipe x impact factor x wheel load
- $(0.006 \times 1.75) (4,032 \text{ lb}) \cong 42 \text{ lb}$.

2.3.2 Maximum Actual Load

- $1193 \text{ lb} + 42 \text{ lb} = 1,235 \text{ lb}$, which is less than 2,000-lb crushing strength. Therefore, no reinforcing is necessary.

Using appropriate values for the 8-in. line, the maximum load is calculated at 1,248 lb, which is less than the 2,000 lb required to crush the pipe (see Attachment 1).

2.4 Evaluation of Other Piping

The steel water lines and fire water lines have allowable working pressures that are in excess of the crushing strength of the 10-in. or 8-in. sanitary sewer lines. The maximum working pressures for these pipes (assumed to be Schedule 40 wrought steel pipe) are 1,400 psi for the 4-in. pipe and 1,100 psi for the 8-in. line, respectively. Because of the working pressure of these lines, no additional evaluation is necessary.

3. OVERHEAD COMMUNICATION AND ELECTRICAL LINES

The height of the transporter and tank is approximately 21.5 ft. The following list summarizes the overhead line heights and required actions.

- Immediately east of the TSF-26 site, power and communication lines cross the path of the transporter at heights of 33.1 ft and 20.8 ft, respectively. The communication line should be raised or removed.
- Two communication lines run south from power pole #54-8 across the transporter path at heights of 19.7 ft and 18.6 ft. Both of these lines will need to be raised or removed.

- A set of power and communication lines run parallel to Snake Avenue. The transport path crosses beneath these lines between power pole #54-8 and #54-7. The power lines are at a height of 40.5 ft and communication lines are located at 20.5 ft, 21.1 ft, 21.7 ft, and 24.5 ft. The three communication lines should be raised or removed.

4. TRANSPORT PATH FILL AND GRADE REQUIREMENTS

Based on site visits conducted by Portage Environmental, Inc., and its sub-tier contractors, only one area of the proposed transport path requires additional fill and grading to support transport of the PM-2A tanks beyond that already specified as part of tank excavation and site preparation activities associated with the TSF-26 site. The area directly west of the TAN-607A High Bay entrance and the access road requires fill and grading to support the turn radius of the transporter to allow the load to be backed into the TAN-607A High Bay.

An area 20 ft in width and 90 ft in length from the edge of the existing asphalt top will need to be filled, graded, and compacted along the railroad tracks. Fill materials shall be placed in loose lifts not exceeding 6 in. in thickness, uniformly moisture-conditioned, and compacted to a minimum of 90% of maximum dry density per ASTM D-698, within 2% of optimum moisture content. The top of the finished grade (ground surface) will be flat and level with the tops of the rails.

5. CONCLUSIONS AND RECOMMENDATIONS

Based on review of the buried piping beneath the transport path, no additional site preparation actions are necessary for the transport of the tanks.

Six overhead communication lines need to be removed or raised to provide adequate entrance during transport of the tanks.

Grade and fill activities are required west of the TAN-607A High Bay.

6. REFERENCES

ACPA, 1970, *Concrete Pipe Design Manual*, American Concrete Pipe Association, 1970.

ACPA, 1974, "Three-Edge Bearing Strengths Nonreinforced Concrete Pipe and Clay Pipe," *Design Data 25*, American Concrete Pipe Association, July 1974.

NCPI, 1968, *Clay Pipe Engineering Manual*, National Clay Pipe Institute, 1968.

Paige, B. E., 1972, *Buried Waste Line Register for NRTS, Part II TAN*, Allied Chemical Corporation, ACI-108, January 1972.

7. DRAWINGS

C-067-RP0003-003, Duratek Drawing, *INEEL PM-2A Tank Site Transportation and Hardware*, Revision 2, May 12, 2004.

1229-5 ANP/GE-3-607-U106, The Ralph M. Parsons Company Drawing, *ANP Assembly & Maintenance Area Expansion of Assembly & Maintenance Bldg. 607 Civil Water-Engine Fuel- Steam-Sanitary Sewer Plans and Profiles*, Revision B, August 3, 1956.

1230 TAN/TSF 305-1, *TAN Area Waste Line Identification*, November 16, 1971.

217515, *Water Line A&M Area*, April 1, 1957.

423185, Revision 9, Sheet 3 of 6.

Attachment I

Design Calculations and Reference Tables

design data 25



THREE-EDGE BEARING STRENGTHS NONREINFORCED CONCRETE PIPE AND CLAY PIPE

Pipe strength classifications established by manufacturing and material standards for nonreinforced concrete pipe and clay pipe are based on three-edge bearing test strengths expressed in pounds per linear foot. These test strengths are directly related to the load carrying capacity of the buried pipe. Although the methods of presenting the strength classes of these two pipe materials are similar, nonreinforced concrete pipe strength classes provide the designer with additional latitude in selecting pipe strengths to satisfy a broader range of performance requirements.

STRENGTH COMPARISONS

The American Society for Testing and Materials (ASTM) Specification C14, Concrete Sewer, Storm Drain and Culvert Pipe, covers the three strength classes of nonreinforced concrete pipe. ASTM Specification C700, Extra Strength and Standard Strength Clay Pipe and Perforated Clay Pipe, covers two strength classifications of clay pipe. Table 1 presents a comparison of the specified minimum three-edge bearing strengths of the three classes of nonreinforced concrete pipe versus the standard strength class and extra strength class of clay pipe through 24 inch diameter. Class 1 minimum three-edge bearing strengths for nonreinforced concrete pipe exceed or meet the minimum three-edge bearing strengths of standard strength clay pipe in each size. Class 3 minimum three-edge bearing strengths for nonreinforced concrete pipe exceed or meet the minimum three-edge bearing strengths of extra strength clay pipe in each size. Class 2 nonreinforced concrete pipe provides the designer with intermediate strengths and the option to select a strength meeting load requirements more realistically and econom-

ically. Figure 1 presents these comparisons in graphic form and enables the designer to readily determine the inherent structural advantages of nonreinforced concrete pipe.

TESTING PROCEDURE

The complete procedure of testing concrete pipe and clay pipe are contained in ASTM Standard C497, Methods of Testing Concrete Pipe and Tile, and ASTM Standard C301, Methods of Testing Clay Pipe. The external load crushing strength test with the load applied by three-edge bearing is the accepted test method for both clay and concrete pipe. The test procedures are similar, except when wooden bearing strips are used for clay pipe plaster of paris must be cast on the contact edges since the barrel of the pipe is often irregular. This same procedure may be employed in the testing of concrete pipe if it is mutually agreed upon by the manufacturer and purchaser prior to testing.

SELECTION OF PIPE STRENGTH

The required three-edge bearing strengths for concrete pipe and clay pipe are computed by the equation:

$$T.E.B. = \frac{W_L + W_E}{L_f} \times F.S. \quad (1)$$

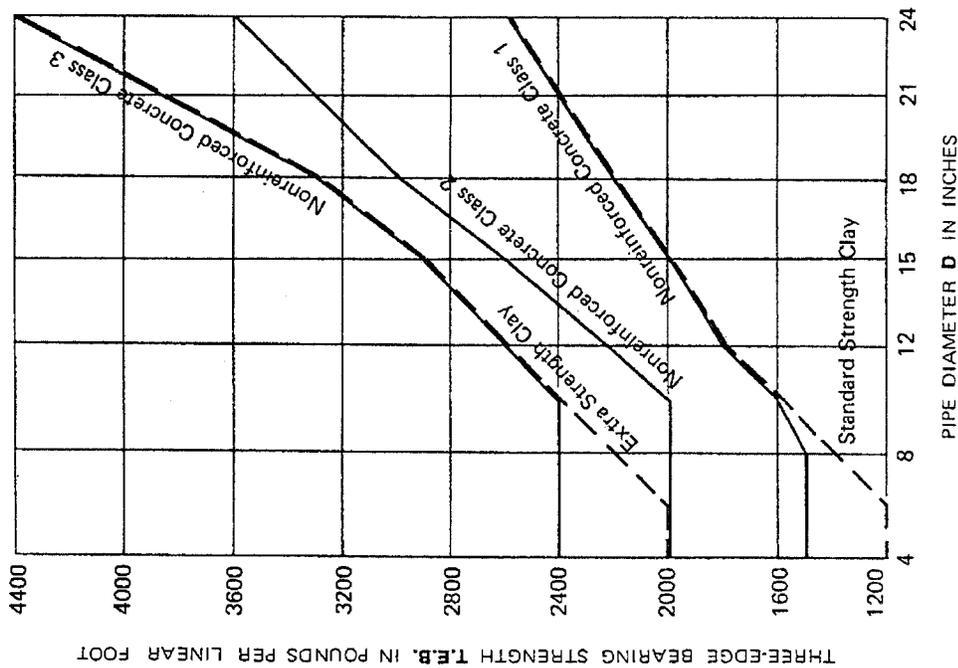
where:

- T.E.B. = three-edge bearing strength, pounds per linear foot
- W_L = live load, pounds per linear foot
- W_E = earth load, pounds per linear foot
- L_f = load factor
- F.S. = factor of safety

TABLE 1: Three-Edge Bearing Strengths — Nonreinforced Concrete, Classes 1, 2, and 3, Clay, Standard Strength and Extra Strength

Diameter	Nonreinforced Concrete			Clay		Diameter
	Class 1	Class 2	Class 3	Standard Strength	Extra Strength	
4	1500	2000	2400	1200	2000	4
6	1500	2000	2400	1200	2000	6
8	1500	2000	2400	1400	2200	8
10	1600	2000	2400	1600	2400	10
12	1800	2250	2600	1800	2600	12
15	2000	2600	2900	2000	2900	15
18	2200	3000	3300	2200	3300	18
21	2400	3300	3850	2400	3850	21
24	2600	3600	4400	2600	4400	24

**FIGURE 1: Strength Comparison
Nonreinforced Concrete Pipe and Clay Pipe**



EXAMPLE 1

Given: A 24-inch diameter sanitary sewer line is to be installed on a Class B bedding with a load factor (L_r) of 1.9, in a trench 17 feet deep, with a factor of safety of 1.5. Live load is negligible. Earth load is 4372 pounds per linear foot.

Find: The required strength class of nonreinforced concrete pipe and clay pipe to carry the field loads.

Solution: Determine the required three-edge bearing test strength from equation (1).

$$\begin{aligned} \text{T.E.B.} &= \left(\frac{W_L}{1.5} + \frac{W_E}{L_r} \right) \text{F.S.} \\ &= \frac{4372}{1.5} \times 1.5 \\ &= 3452 \text{ pounds per linear foot} \end{aligned}$$

Using Table 1, locate 24-inch diameter in appropriate column. Proceeding to the right determine most economical class of nonreinforced concrete pipe that exceeds the 3452 T.E.B. strength required. Follow same procedure to determine required class of clay pipe.

Answer: A Class 2 nonreinforced concrete pipe or an extra strength clay pipe is required.

The required strength class could also have been obtained from Figure 1.

EXAMPLE 2

Given: A 12-inch sanitary sewer line is to be installed in a 2½-foot wide trench with 13 feet of cover over the top of the pipe. The pipe will be back-filled with ordinary clay weighing 120 pounds per cubic foot. A Class C bedding is specified and a factor of safety of 1.5 will be used. Live load is negligible and earth load is 2137 pounds per linear foot.

Find: The required strength class of nonreinforced concrete pipe and clay pipe to carry the field loads.

Solution: Determine the required three-edge bearing test strength from equation (1).

$$\begin{aligned} \text{T.E.B.} &= \left(\frac{W_L}{1.5} + \frac{W_E}{L_r} \right) \text{F.S.} \\ &= \frac{2137}{1.5} \times 1.5 \\ &= 2137 \text{ pounds per linear foot} \end{aligned}$$

Enter Figure 1 at D = 12 inches on the horizontal scale and project a vertical line to the nonreinforced concrete pipe class and clay pipe class which has a three-edge bearing strength greater than 2137 as indicated on the vertical scale.

Answer: A Class 2 nonreinforced concrete pipe or an extra strength clay pipe is required.

The required strength class could also have been obtained from Table 1.

* maximum live load factor of 1.5 is recommended

**PERCENTAGE OF WHEEL LOADS
TRANSMITTED TO UNDERGROUND PIPES***

Tabulated figures show percentage of wheel load applied to one lineal foot of pipe.

Depth of Backfill Over Top of Pipe in feet	Pipe Size Inches	6"	8"	10"	12"	15"	18"	21"	24"	27"	30"	33"	36"	39"	42"
		Outside Diam. of Pipe in Feet (Approx.)	.64	.81	1.0	1.2	1.5	1.8	2.1	2.4	2.7	3.0	3.3	3.5	3.9
1		12.8	15.0	17.3	20.0	22.6	24.8	26.4	27.2	28.0	28.6	29.0	29.4	29.8	29.9
2		5.7	7.0	8.3	9.6	11.5	13.2	15.0	15.6	16.8	17.8	18.7	19.5	20.0	20.5
3		2.9	3.6	4.3	5.2	6.4	7.5	8.6	9.3	10.2	11.1	11.8	12.5	12.9	13.5
4		1.7	2.1	2.5	3.1	3.9	4.6	5.3	5.8	6.5	7.2	7.9	8.5	8.8	9.2
5		1.2	1.4	1.7	2.1	2.6	3.1	3.6	3.9	4.4	4.9	5.3	5.8	6.1	6.4
6		0.8	1.0	1.1	1.4	1.8	2.1	2.5	2.8	3.1	3.5	3.8	4.2	4.3	4.4
7		0.5	0.7	0.8	1.0	1.3	1.6	1.9	2.1	2.3	2.6	2.9	3.2	3.3	3.5
8		0.4	0.5	0.6	0.8	1.0	1.2	1.4	1.6	1.8	2.0	2.2	2.3	2.5	2.6

*These figures make no allowance for impact. For moving loads (particularly construction equipment, etc.) on unsurfaced areas the percentage figures above should be multiplied by an impact factor of 1.5 Highway, 1.75 Railway, 1.00 Airfield Runways, 1.50 Airfield Taxiways.

Unless other data are available, it is safe to estimate that truck wheel loads are the greatest live loads to be supported. H-20 wheel loadings are standard for highway and bridge design and are equally applicable for estimating live loads on sewers.

H-20 refers to loadings resulting from the passage of trucks having a gross weight of 20 tons, 80% of which is on the rear axle, with axle spacing of 14'0" center to center, and a wheel gauge of 6'0", each rear wheel carrying one half this load or 16,000 pounds each without impact.

$$W_{us} = C_{us} B_d U_s$$

W_{us} = The average total vertical load on a section of a conduit due to U_s , pounds per linear foot.

C_{us} = A coefficient for superimposed loads on ditch conduits, abstract number. (See table on p. 41.)

B_d = Horizontal breadth of ditch at top of conduit, feet.

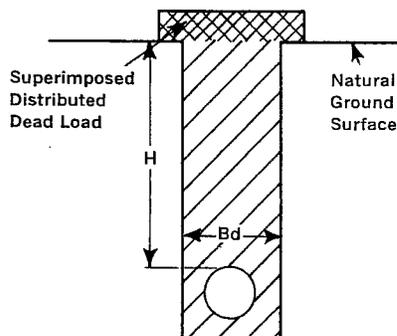
U_s = uniformly distributed, superimposed load, pounds per square foot.

H = Vertical height from top of conduit to upper surface of fill, feet.

Superimposed Loads

Sewers located in yards of industrial plants or under earth fills may have to withstand superimposed loads in addition to trench backfill loads.

Where superimposed loads are encountered they are added to the pre-determined backfill load. To compute the superimposed load on a pipe the following Marston formula is used.



load analysis on 10" Sanitary Sewer

Wheel Load = 4032 #/ft²
Assume Soil Dry Clay 120 #/ft³

OD = 1'-0" D = 8.21'

Live Load:

Impact factor Range 1.0 to 1.75

Use 1.75 (most conservative)

90% of wheel load to reach pipe

= 0.006 from Table Page 40
Clay Pipe Engrg Manual

From Table 14 Conc Pipe
Design Manual

Ordinary clay @ 8' depth

Maximum Value = 1193 #/ft

$$\text{Max Actual Load} = (0.006 \times 1.75 / 4032)^{\#} + 1193 \#/\text{ft}$$

$$= 1235 \#/\text{ft}$$

Bearing strength of 10" Conc Pipe from
ACPI Design Data 25 = 2000 #

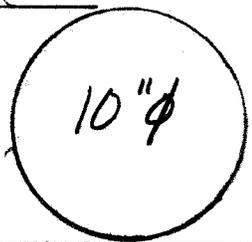
$$1235 \#/\text{ft} < 2000 \#/\text{ft}$$

13-782 500 SHEETS, FILLER 5 SQUARE
42-381 50 SHEETS, RELEASE 5 SQUARE
42-380 100 SHEETS, RELEASE 5 SQUARE
42-389 200 SHEETS, RELEASE 5 SQUARE



Made in U.S.A.

8.21'



Load analysis on 8" Sanitary Sewer

OD = 0.8125' D = 9.1875'

Wheel load = 4032 #/ft²

Assume Soil Dry Clay 120 #/ft³

Live Load:

Impact factor Range 1.0 to 1.75 Use 1.75
reasons stated previously. (most conservative)

% of wheel load to reach pipe = 0.5%
from Table page 40 Clay Pipe Eng'g Manual.

From Table 13 Concrete Pipe Design Manual
Max value for 10' depth = 1213 #/ft

Max Actual load = Impact Load + Backfill Load
= (0.005 x 1.75)(4032) #/ft² + 1213 #/ft
= 1248.28 #/ft

Bearing Strength of 8" conc pipe from
ACPI Design Data 25 = 2000 #

1248 #/ft < 2000 #/ft

13-782 500 SHEETS, FILLER, 5 SQUARE
42-381 50 SHEETS EYE-EASE, 5 SQUARE
42-382 100 SHEETS EYE-EASE, 5 SQUARE
42-383 200 SHEETS EYE-EASE, 5 SQUARE

Made in U.S.A.



TABLE 14

10"

BACKFILL LOADS ON CIRCULAR PIPE IN TRENCH INSTALLATION

*100 POUNDS PER CUBIC FOOT BACKFILL MATERIAL

LOADS IN POUNDS PER LINEAR FOOT

10"

A SAND AND GRAVEL $K_{\mu}' = -0.165$

HEIGHT OF BACKFILL H ABOVE TOP OF PIPE, FEET	TRENCH WIDTH AT TOP OF PIPE										TRANSITION WIDTH
	1'-9"	2'-0"	2'-3"	2'-6"	2'-9"	3'-0"	3'-3"	3'-6"	4'-0"	4'-6"	
5	566	680	743								2'-2"
6	628	761	893								2'-3"
7	680	830	984	1043							2'-4"
8	722	888	1059	1193							2'-5"
9	758	937	1124	1344							2'-6"
10	787	979	1180	1388	1495						2'-8"
11	811	1014	1228	1450	1645						2'-8"
12	831	1044	1270	1505	1748	1795					2'-10"
13	848	1070	1306	1553	1810	1946					2'-11"
14	861	1091	1337	1595	1864	2094					3'-0"
15	873	1110	1364	1632	1912	2241					3'-0"
16	882	1125	1387	1664	1955	2258	2395				3'-1"
17	890	1138	1407	1693	1993	2306	2547				3'-2"
18	896	1149	1424	1717	2027	2350	2698				3'-3"
19	902	1159	1439	1739	2057	2389	2735	2842			3'-4"
20	906	1167	1452	1758	2083	2425	2780	2994			3'-5"
21	910	1174	1463	1775	2107	2456	2821	3150			3'-6"
22	913	1179	1473	1790	2128	2484	2857	3301			3'-6"
23	915	1184	1481	1802	2146	2510	2891	3325	3445		3'-7"
24	917	1189	1488	1814	2163	2532	2920	3325	3595		3'-8"
25	919	1192	1494	1824	2177	2552	2947	3360	3739		3'-9"
26	921	1195	1500	1832	2190	2571	2972	3392	3892		3'-9"
27	922	1198	1504	1840	2201	2587	2994	3421	4041		3'-10"
28	923	1200	1508	1846	2212	2601	3014	3447	4201		3'-11"
29	924	1201	1512	1852	2221	2614	3032	3471	4340		4'-0"
30	924	1203	1515	1857	2229	2626	3048	3492	4493		4'-0"
31	925	1204	1517	1862	2236	2637	3063	3512	4472	4642	4'-1"
32	925	1205	1520	1866	2242	2646	3076	3530	4502	4786	4'-2"
33	926	1206	1521	1869	2247	2654	3088	3546	4529	4950	4'-2"
34	926	1207	1523	1872	2252	2662	3099	3561	4555	5085	4'-3"
35	926	1208	1525	1875	2257	2669	3109	3575	4578	5243	4'-4"
36	926	1208	1526	1877	2261	2675	3118	3587	4599	5397	4'-4"
37	927	1209	1527	1879	2264	2680	3126	3598	4619	5549	4'-5"
38	927	1209	1528	1881	2267	2685	3133	3608	4637	5697	4'-6"
39	927	1210	1529	1882	2270	2689	3139	3618	4654	5842	4'-6"
40	927	1210	1529	1884	2272	2693	3145	3626	4669	5983	4'-7"

B SATURATED TOP SOIL $K_{\mu}' = -0.150$

HEIGHT OF BACKFILL H ABOVE TOP OF PIPE, FEET	TRENCH WIDTH AT TOP OF PIPE										TRANSITION WIDTH
	1'-9"	2'-0"	2'-3"	2'-6"	2'-9"	3'-0"	3'-3"	3'-6"	4'-0"	4'-6"	
5	587	703	743								2'-1"
6	655	791	893								2'-2"
7	713	866	1043								2'-3"
8	761	931	1106	1193							2'-4"
9	802	987	1179	1344							2'-5"
10	836	1035	1242	1455	1495						2'-6"
11	865	1077	1298	1526	1645						2'-7"
12	890	1112	1346	1589	1795						2'-8"
13	910	1143	1389	1645	1946						2'-9"
14	928	1170	1426	1695	1973	2094					2'-10"
15	942	1192	1459	1738	2030	2241					2'-11"
16	955	1212	1487	1777	2080	2395					3'-0"
17	965	1229	1512	1812	2126	2451	2547				3'-1"
18	974	1243	1534	1843	2167	2504	2698				3'-2"
19	981	1256	1553	1870	2203	2551	2842				3'-3"
20	987	1266	1570	1894	2236	2593	2994				3'-4"
21	992	1276	1584	1915	2265	2632	3014	3150			3'-5"
22	997	1284	1597	1934	2292	2667	3058	3301			3'-6"
23	1001	1291	1608	1951	2315	2699	3099	3445			3'-7"
24	1004	1296	1618	1966	2336	2727	3136	3595			3'-8"
25	1006	1301	1627	1979	2355	2753	3170	3604	3739		3'-9"
26	1008	1306	1634	1991	2373	2777	3201	3643	3892		3'-10"
27	1010	1310	1641	2001	2388	2798	3229	3679	4041		3'-11"
28	1012	1313	1647	2010	2401	2817	3255	3712	4201		4'-0"
29	1013	1316	1652	2019	2414	2834	3278	3743	4340		4'-0"
30	1014	1318	1656	2026	2425	2850	3300	3771	4493		4'-1"
31	1015	1320	1660	2032	2435	2864	3319	3796	4642		4'-2"
32	1016	1322	1663	2038	2444	2877	3337	3820	4786		4'-2"
33	1017	1323	1666	2043	2451	2889	3353	3842	4950		4'-3"
34	1017	1325	1669	2048	2459	2899	3368	3861	4916	5085	4'-4"
35	1018	1326	1671	2052	2465	2909	3381	3380	4946	5243	4'-5"
36	1018	1327	1673	2055	2471	2918	3393	3896	4974	5397	4'-6"
37	1019	1328	1675	2058	2476	2925	3405	3912	5000	5549	4'-7"
38	1019	1328	1676	2061	2480	2932	3415	3926	5024	5697	4'-8"
39	1019	1329	1678	2064	2485	2939	3424	3939	5047	5842	4'-9"
40	1019	1330	1679	2066	2488	2945	3433	3950	5067	5983	4'-10"

C ORDINARY CLAY $K_{\mu}' = -0.130$

HEIGHT OF BACKFILL H ABOVE TOP OF PIPE, FEET	TRENCH WIDTH AT TOP OF PIPE										TRANSITION WIDTH
	1'-9"	2'-0"	2'-3"	2'-6"	2'-9"	3'-0"	3'-3"	3'-6"	4'-0"	4'-6"	
5	617	743									2'-0"
6	694	833	893								2'-1"
7	761	919	1043								2'-2"
8	819	994	1193								2'-3"
9	868	1060	1258	1344							2'-4"
10	911	1119	1334	1495							2'-5"
11	948	1170	1400	1645							2'-6"
12	979	1215	1460	1713	1795						2'-7"
13	1007	1254	1513	1781	1946						2'-8"
14	1030	1289	1560	1843	2094						2'-8"
15	1051	1319	1603	1898	2241						2'-9"
16	1068	1346	1640	1948	2267	2395					2'-10"
17	1083	1369	1674	1993	2325	2547					2'-11"
18	1096	1390	1703	2034	2378	2698					3'-0"
19	1107	1408	1730	2070	2426	2842					3'-0"
20	1117	1424	1754	2103	2469	2849	2994				3'-1"
21	1125	1438	1775	2133	2509	2900	3150				3'-2"
22	1133	1450	1793	2159	2545	2947	3301				3'-2"
23	1139	1461	1810	2184	2578	2989	3445				3'-3"
24	1144	1470	1825	2205	2607	3029	3466	3595			3'-4"
25	1149	1478	1838	2225	2635	3064	3512	3739			3'-5"
26	1153	1486	1850	2242	2659	3097	3554	3892			3'-5"
27	1156	1492	1861	2258	2682	3128	3593	4041			3'-6"
28	1159	1498	1870	2273	2702	3155	3630	4201			3'-6"
29	1162	1502	1878	2286	2721	3181	3663	4165	4340		3'-7"
30	1164	1507	1886	2297	2738	3204	3693	4204	4493		3'-8"
31	1166	1511	1892	2308	2753	3225	3722	4240	4642		3'-8"
32	1167	1514	1898	2317	2767	3245	3748	4274	4786		3'-9"
33	1169	1517	1904	2326	2780	3263	3772	4305	4950		3'-9"
34	1170	1519	1908	2333	2791	3279	3794	4334	5085		3'-10"
35	1171	1522	1913	2340	2802	3294	3815	4361	5243		3'-11"
36	1172	1524	1916	2346	2811	3308	3834	4386	5397		3'-11"
37	1173	1525	1920	2352	2820	3321	3851	4409	5549		4'-0"
38	1173	1527	1922	2357	2828	3333	3868	4431	5697		4'-0"
39	1174	1528	1925	2362	2835	3343	3883	4451	5666	5842	4'-1"
40	1174	1529	1927	2366	2842	3353	3896	4470	5696	5983	4'-1"

Handwritten note: "MAXIMUM VALUE" with a circle around the value 1193 in the 8-foot height row, 2'-3" trench width column.

D SATURATED CLAY $K_{\mu}' = -0.110$

HEIGHT OF BACKFILL H ABOVE TOP OF PIPE, FEET	TRENCH WIDTH AT TOP OF PIPE										TRANSITION WIDTH
	1'-9"	2'-0"	2'-3"	2'-6"	2'-9"	3'-0"	3'-3"	3'-6"	4'-0"	4'-6"	
5	649	743									1'-11"
6	737	893									2'-0"
7	814	976	1043								2'-1"
8	882	1064	1193								2'-2"
9	943	1142	1344								2'-3"
10	996	1212	1435	1495							2'-4"
11	1042	1276	1516	1645							2'-5"
12	1084	1332	1589	1795							2'-6"
13	1120	1383	1655	1946							2'-7"
14	1152	1428	1715	2012	2094						2'-8"
15	1180	1469	1770	2082	2241						

Welded and Seamless Wrought Steel Pipe

Size	Schedule no	Diameter		Thick-ness	Circumference		Transverse area		Length of pipe per sq ft of surface area		Weight per ft of length	Allowable working pressure at 100°F*	Water hammer factor
		External	Internal		External	Internal	External	Internal	External surface	Internal surface			
		inches	inches		inches	inches	sq in	sq in	feet	feet			
3	40 -S	3.500	3.068	0.216	10.996	9.638	9.621	7.393	1.091	1.245	7.58	1600	2.60
	80 -X		2.900		9.111	9.621	6.605	1.091	1.32	10.25	2600	2.92	
	120		2.624		10.996	9.621	5.408	1.091	1.46	14.32	4100	3.56	
	160 XX		2.300		10.996	9.621	4.155	1.091	1.66	18.58	6100	4.64	
3½	40 -S	4.000	3.548	0.226	12.566	11.146	12.566	9.886	0.954	1.076	9.11	1500	1.94
	80 -X		3.364		12.566	10.57	8.888	0.954	1.14	12.50	2400	2.17	
	120		2.728		12.566	8.57	12.566	0.954	1.40	15.00	3350	2.82	
	160 XX		2.300		12.566	8.57	12.566	0.954	1.40	15.00	3350	2.82	
4	40 -S	4.500	4.026	0.237	14.137	12.648	15.904	12.703	0.848	0.948	10.79	1400	1.51
	80 -X		3.826		14.137	12.020	11.497	0.848	0.998	14.98	2300	1.67	
	120		3.624		14.137	11.39	15.904	0.848	1.05	19.00	3350	1.87	
	160 XX		3.438		14.137	10.80	15.904	0.848	1.11	22.51	4000	2.08	
5	40 -S	5.563	5.047	0.258	17.477	15.856	24.306	20.006	0.686	0.756	14.62	1300	0.960
	80 -X		4.813		17.477	15.120	18.19	0.686	0.793	20.78	2090	1.06	
	120		4.563		17.477	14.34	24.306	0.686	0.837	27.04	2950	1.19	
	160 XX		4.313		17.477	13.55	24.306	0.686	0.897	32.96	3850	1.32	
6	40 -S	6.625	6.065	0.280	20.813	19.054	34.472	28.891	0.756	0.629	18.97	1210	0.666
	80 -X		5.761		20.813	18.099	26.07	0.756	0.663	28.57	2070	0.738	
	120		5.501		20.813	17.29	34.472	0.756	0.695	36.39	2850	0.810	
	160 XX		5.187		20.813	16.30	34.472	0.756	0.736	45.35	3760	0.912	
8	20	8.625	8.125	0.250	27.096	25.53	58.43	51.87	0.443	0.470	22.36	795	0.371
	30		8.071		27.096	25.39	58.43	0.443	0.473	24.70	910	0.375	
	40 S		7.981		27.096	25.07	58.43	0.443	0.478	28.55	1100	0.385	
	60		7.813		27.096	24.54	58.43	0.443	0.489	35.64	1460	0.402	
	80 X		7.625		27.096	23.955	58.43	0.443	0.500	43.39	1870	0.422	
	100		7.437		27.096	23.36	58.43	0.443	0.514	50.95	2280	0.443	
	120		7.187		27.096	22.58	58.43	0.443	0.532	60.71	2840	0.475	
	140		7.001		27.096	21.99	58.43	0.443	0.546	67.76	3260	0.500	
	160 XX		6.875		27.096	21.60	58.43	0.443	0.556	72.42	3560	0.519	
	160 XX		6.813		27.096	21.40	58.43	0.443	0.561	74.69	3700	0.529	

Selected from ANSI B 36.10—1975. See notes, page 7-19.

Welded and Seamless Wrought Steel Pipe (Continued)

INGERSOLL-RAND CAMERON HYDRAULIC DATA

Welded and Seamless Wrought Steel Pipe

Size	Schedule no	Diameter		Thick-ness	Circumference		Transverse area		Length of pipe per sq ft of surface area		Weight per ft of length	Allowable working pressure at 100°F*	Water hammer factor	
		External	Internal		External	Internal	External	Internal	External surface	Internal surface				
		inches	inches		inches	inches	sq in	sq in	feet	feet				
10	20	10.750	10.250	0.250	33.77	32.20	90.76	82.52	0.355	0.373	28.04	636	233	
	30		10.136		0.307	33.77	31.84	90.76	80.69	0.355	0.377	34.24	827	239
	40 S		10.020		0.365	33.77	31.48	90.76	78.85	0.355	0.381	40.48	1030	244
	60 X		9.750		0.500	33.77	30.63	90.76	74.66	0.355	0.392	54.74	1490	256
	80		9.562		0.594	33.77	30.04	90.76	71.81	0.355	0.399	64.43	1800	268
	100		9.312		0.719	33.77	29.25	90.76	68.10	0.355	0.410	77.03	2250	283
	120		9.062		0.844	33.77	28.48	90.76	64.50	0.355	0.421	89.29	2700	299
	160 XX		8.750		1.000	33.77	27.49	90.76	60.13	0.355	0.437	104.13	3310	320
160 XX	8.500	1.125	33.77	26.70	90.76	56.75	0.355	0.449	115.64	3740	340			
12	20	12.750	12.250	0.250	40.06	38.48	127.68	117.86	0.299	0.312	33.38	535	163	
	30		12.090		0.330	40.06	37.98	127.68	114.80	0.299	0.316	43.77	760	168
	40 S		11.938		0.375	40.06	37.70	127.68	113.10	0.299	0.318	49.56	890	170
	60 X		11.938		0.406	40.06	37.50	127.68	111.93	0.299	0.320	53.52	1000	172
	80		11.750		0.500	40.06	36.91	127.68	108.43	0.299	0.325	65.42	1250	178
	100		11.626		0.626	40.06	36.52	127.68	106.16	0.299	0.329	73.15	1425	181
	120		11.374		0.698	40.06	35.73	127.68	101.61	0.299	0.336	88.63	1800	190
	140		11.062		0.844	40.06	34.75	127.68	96.11	0.299	0.345	107.32	2255	200
	160 XX		10.750		1.000	40.06	33.77	127.68	90.76	0.299	0.355	125.49	2700	212
	160 XX		10.500		1.125	40.06	32.99	127.68	86.59	0.299	0.364	139.67	3115	222
	160 XX		10.126		1.312	40.06	31.81	127.68	80.53	0.299	0.377	160.27	3700	239
	14		10		14.000	13.500	0.250	43.98	42.41	153.94	143.14	0.272	0.283	36.71
20		13.376	0.312	43.98		42.02		153.94	140.52	0.272	0.286	45.61	645	137
30 S		13.250	0.375	43.98		41.63		153.94	137.89	0.272	0.288	54.57	810	138
40 X		13.124	0.438	43.98		41.23		153.94	135.28	0.272	0.291	63.44	970	142
60		13.000	0.500	43.98		40.84		153.94	132.73	0.272	0.294	72.09	1140	145
80		12.812	0.594	43.98		40.25		153.94	128.92	0.272	0.298	85.05	1380	149
100		12.500	0.750	43.98		39.26		153.94	122.72	0.272	0.306	106.13	1795	157
120		12.124	0.938	43.98		38.09		153.94	115.45	0.272	0.315	130.85	2300	167
140		11.876	1.062	43.98		37.31		153.94	110.77	0.272	0.322	150.79	2645	174
160 XX		11.500	1.250	43.98		36.13		153.94	103.87	0.272	0.332	170.21	3170	185
160 XX		11.188	1.406	43.98		35.15		153.94	93.31	0.272	0.341	189.11	3615	206

Selected from ANSI B 36.10—1975. See notes, page 7-19.

Welded and Seamless Wrought Steel Pipe (Continued)

CONVERSION FACTOR DATA

CONCRETE PIPE DESIGN MANUAL

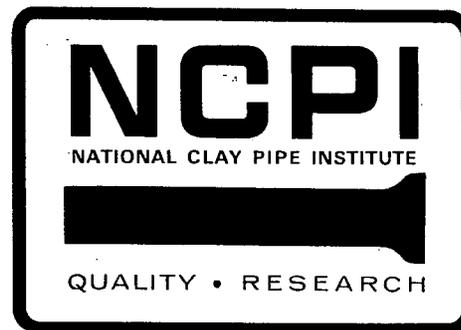
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