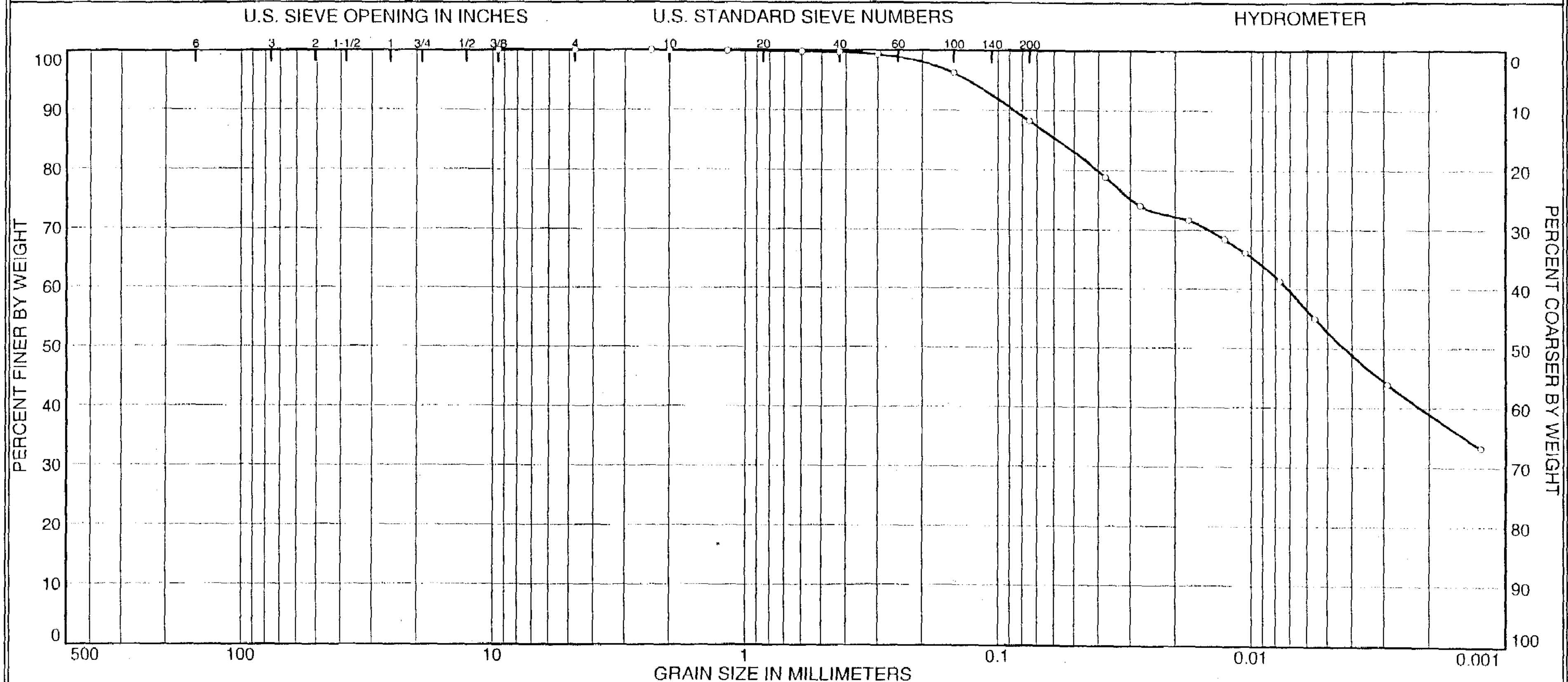
THE CONTENTS OF THIS SECTION ARE
THE HIGHEST QUALITY AVAILABLE

INITIAL KH DATE 8/10/04

PARTICLE SIZE DISTRIBUTION TEST REPORT



	% + 3"	% GRAVEL		% SAND			% FINES	
		COARSE	FINE	COARSE	MEDIUM	FINE	SILT	CLAY
○	0.0	0.0	0.0	0.1	0.2	11.5	35.4	52.8

	SOURCE	SAMPLE #	DEPTH/ELEV.	DATE SAMPLED	USCS	MATERIAL DESCRIPTION	NM %	LL	PL
○	D&D TSF 06-26 soils		8.5' - 9.0'	6/10/03	CL	Lean clay	17.8%	33.4	17.0

Client

Project

Project No. Plate

INEEL MATERIALS LAB

○ Sample 1RA17603PR sampled May 14th, 2003. Borehole #3 South.

Lab Log #026

GRAIN SIZE DISTRIBUTION TEST DATA

Client:
Project:
Project Number:

Sample Data

Source: D&D TSF 06-26 soils
Sample No.: 1RA17603PR
Elev. or Depth: 8.5' - 9.0' **Sample Length (in./cm.):** LL #026
Location:
Description: Lean clay
Date: 6/10/03 **Natural Moisture:** 17.8%
Liquid Limit: 33.4 **Plastic Limit:** 17.0 **USCS Class.:** CL
Testing Remarks: Sample 1RA17603PR sampled May 14th, 2003. Borehole #3 South.

Lab Log #026

Mechanical Analysis Data

	Initial
Dry sample and tare=	477.22
Tare =	113.95
Dry sample weight =	363.27

Sieve tare method

Sieve	Weight retained	Sieve tare	Percent finer
3/8 inch	0.00	0.00	100.0
4	0.00	0.00	100.0
# 8	0.00	0.00	100.0
# 10	0.28	0.00	99.9
# 16	0.02	0.00	99.9
# 30	0.27	0.00	99.8
# 40	0.51	0.00	99.7
# 50	1.41	0.00	99.3
# 100	11.19	0.00	96.2
# 200	29.05	0.00	88.2

Hydrometer Analysis Data

Separation sieve is #10
Percent -#10 based upon complete sample= 99.9
Weight of hydrometer sample: 66.26
Hygroscopic moisture correction:
  Moist weight & tare = 482.05
  Dry weight & tare = 477.22
  Tare = 113.95
  Hygroscopic moisture= 1.3 %
Calculated biased weight= 65.46
Automatic temperature correction
  Composite correction at 20 deg C = -4.0

Meniscus correction only= 1.0
Specific gravity of solids= 2.50
Specific gravity correction factor= 1.038
Hydrometer type: 152H
  Effective depth L= 16.294964 - 0.164 x Rm

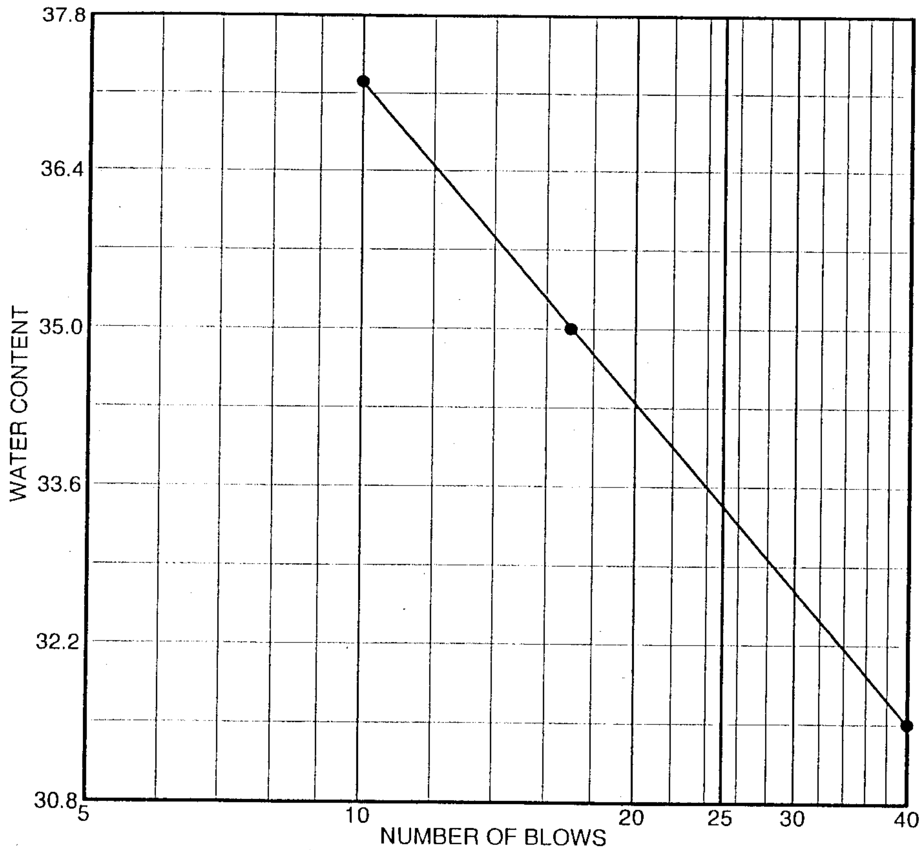
Elapsed time, min	Temp, deg C	Actual reading	Corrected reading	K	Rm	Eff. depth	Diameter mm	Percent finer
1.00	23.0	53.0	49.7	0.0138	54.0	7.4	0.0376	78.8
2.00	23.0	50.0	46.7	0.0138	51.0	7.9	0.0275	74.0
5.00	23.0	48.5	45.2	0.0138	49.5	8.2	0.0176	71.6
10.00	23.0	46.5	43.2	0.0138	47.5	8.5	0.0127	68.4
15.00	23.0	45.0	41.7	0.0138	46.0	8.8	0.0105	66.1
30.00	23.0	42.0	38.7	0.0138	43.0	9.2	0.0077	61.3
60.00	23.0	38.0	34.7	0.0138	39.0	9.9	0.0056	55.0
250.00	23.0	31.0	27.7	0.0138	32.0	11.0	0.0029	43.9
1440.00	24.0	24.0	21.0	0.0136	25.0	12.2	0.0013	33.2

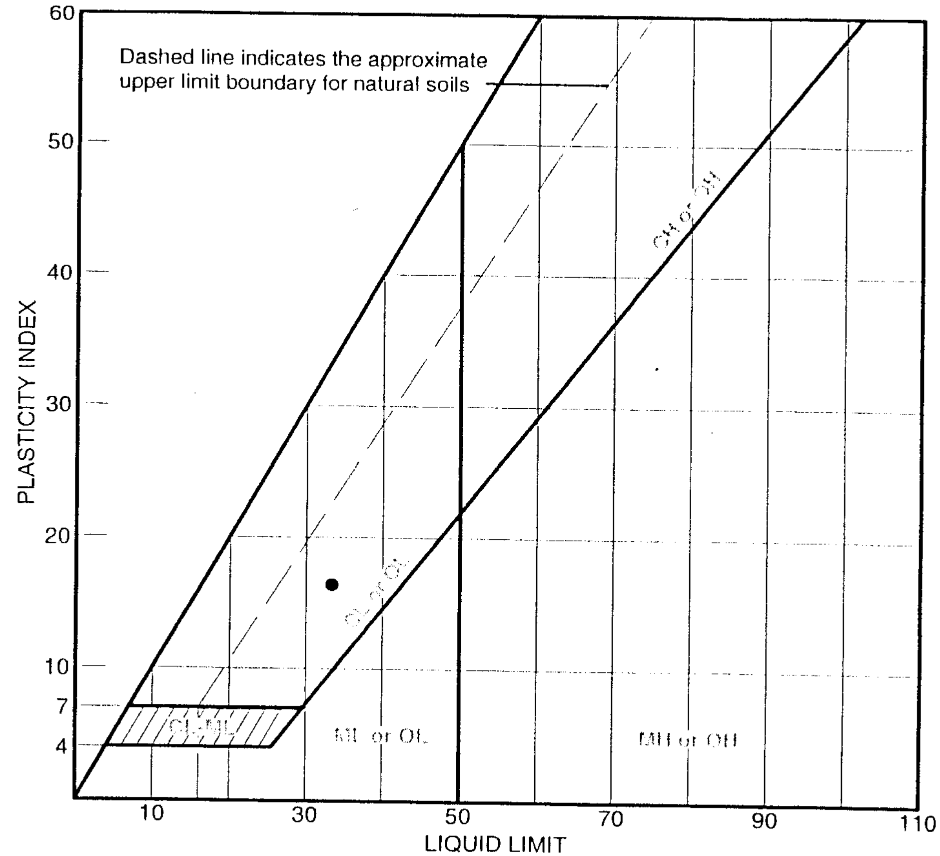
Fractional Components

Gravel/Sand based on #4
Sand/Fines based on #200
% + 3" = **% GRAVEL =**
% SAND = 11.8 (**% coarse =** 0.1 **% medium =** 0.2 **% fine =** 11.5)
% SILT = 35.4 **% CLAY =** 52.8

D_{85}= 0.06 D_{60}= 0.01 D_{50}= 0.00

LIQUID AND PLASTIC LIMITS TEST REPORT





	SOURCE	SAMPLE #	DEPTH/ELEV.	DATE SAMPLED	USCS	MATERIAL DESCRIPTION	NM %	LL	PI
●	D&D TSF 06-26 soils		8.5' - 9.0'	6/10/03	CL	Lean clay	17.8%	33.4	16.4

Client

Project

Project No. Plate

INEEL MATERIALS LAB

● Sample #1RA17603PR sampled May 14th, 2003. Borehole #3 South.
Lab Log #026

LIQUID AND PLASTIC LIMIT TEST DATA

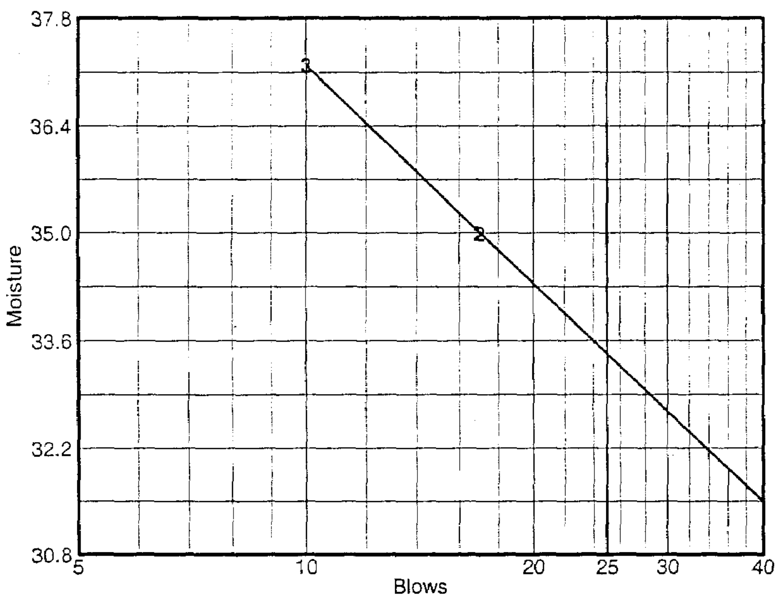
Client:
Project:
Project Number:

Sample Data

Source: D&D TSF 06-26 soils
Sample No.: 1RA17603PR
Elev. or Depth: 8.5' - 9.0'   **Sample Length (in./cm.):** LL #026
Location:
Description: Lean clay
Date: 6/10/03   **Natural Moisture:** 17.8%
USCS Class.: CL   **AASHTO Class.:** A-6(13)
Testing Remarks: Sample #1RA17603PR sampled May 14th, 2003. Borehole #3 South. Lab Log #026

Liquid Limit Data

Run No.	1	2	3	4	5	6
Wet+Tare	24.03	23.48	25.11			
Dry+Tare	20.94	20.28	21.32			
Tare	11.13	11.13	11.14			
# Blows	40	17	10			
Moisture	31.5	35.0	37.2			

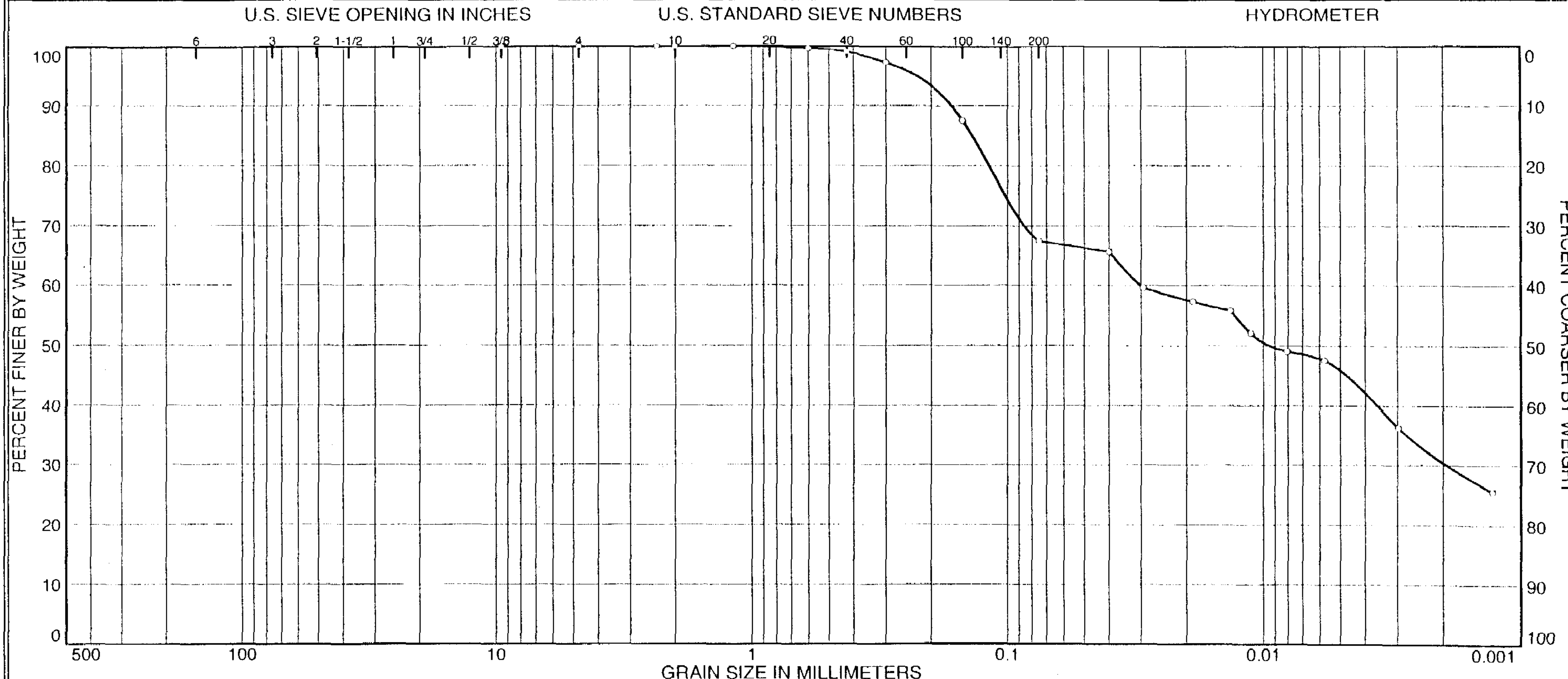


Liquid Limit= 33.4
Plastic Limit= 17.0
Plasticity Index= 16.4

Plastic Limit Data

Run No.	1	2	3	4
Wet+Tare	10.88	9.58		
Dry+Tare	9.93	8.81		
Tare	4.34	4.32		
Moisture	17.0	17.1		

PARTICLE SIZE DISTRIBUTION TEST REPORT



	% + 3"	% GRAVEL		% SAND			% FINES	
		COARSE	FINE	COARSE	MEDIUM	FINE	SILT	CLAY
○	0.0	0.0	0.0	0.0	1.0	31.6	21.4	46.0

	SOURCE	SAMPLE #	DEPTH/ELEV.	DATE SAMPLED	USCS	MATERIAL DESCRIPTION	NM %	LL	PL
○	D&D TSF 06-26 soils		11.0' - 12.5'	6/10/03	CL	Sandy lean clay	14.9%	25.1	14.1

Client

Project

Project No. Plate

INEEL MATERIALS LAB

○ Sample #1RA17604PR sampled May 14th, 2003. Borehole #4 South.

Lab Log #027

GRAIN SIZE DISTRIBUTION TEST DATA

Client:
Project:
Project Number:

Sample Data

Source: D&D TSF 06-26 soils
Sample No.: 1RA17604PR
Elev. or Depth: 11.0' - 12.5' **Sample Length (in./cm.):** LL #027
Location:
Description: Sandy lean clay
Date: 6/10/03 **Natural Moisture:** 14.9%
Liquid Limit: 25.1 **Plastic Limit:** 14.1 **USCS Class.:** CL
Testing Remarks: Sample #1RA17604PR sampled May 14th, 2003. Borehole #4 South.

Lab Log #027

Mechanical Analysis Data

	Initial
Dry sample and tare =	373.95
Tare =	104.87
Dry sample weight =	269.08

Sieve tare method

Sieve	Weight retained	Sieve tare	Percent finer
3/8 inch	0.00	0.00	100.0
# 4	0.00	0.00	100.0
8	0.00	0.00	100.0
# 10	0.00	0.00	100.0
# 16	0.08	0.00	100.0
# 30	0.96	0.00	99.6
# 40	1.66	0.00	99.0
# 50	4.96	0.00	97.2
# 100	25.91	0.00	87.5
# 200	54.23	0.00	67.4

Hydrometer Analysis Data

Separation sieve is #10
Percent -#10 based upon complete sample= 100.0
Weight of hydrometer sample: 70.41
Hygroscopic moisture correction:
Moist weight & tare = 378.96
Dry weight & tare = 373.95
Tare = 104.87
Hygroscopic moisture= 1.9 %
Calculated biased weight= 69.12
Automatic temperature correction
Composite correction at 20 deg C = -4.0

Meniscus correction only= 1.0
S cific gravity of solids= 2.50
S. cific gravity correction factor= 1.038
Hydrometer type: 152H
Effective depth L= 16.294964 - 0.164 x Rm

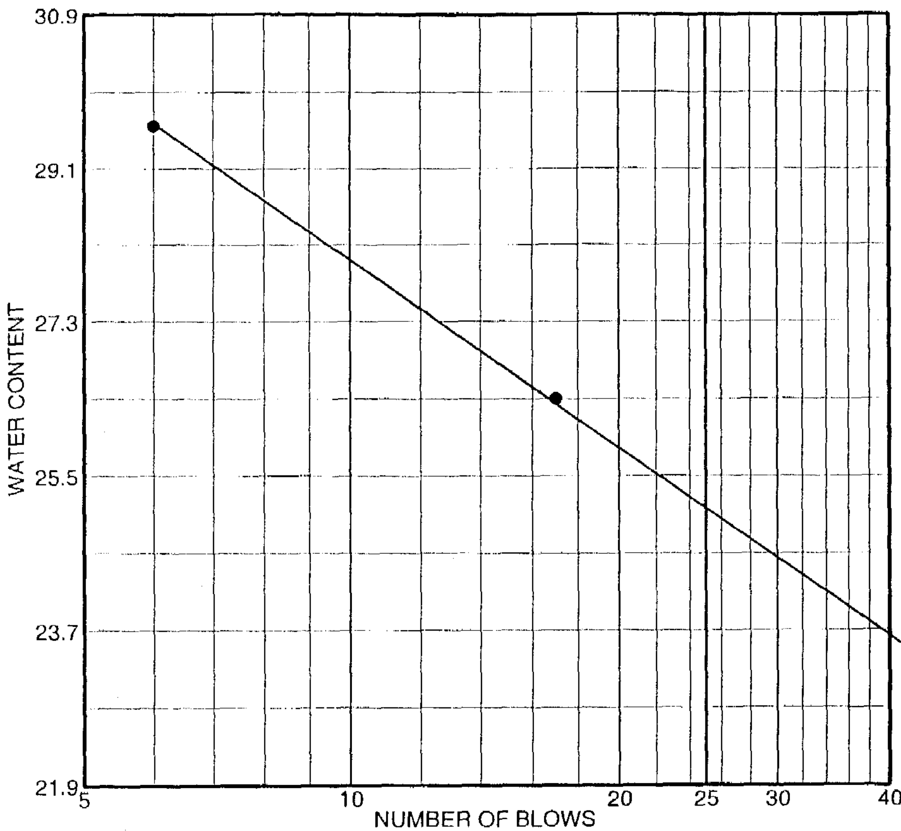
Elapsed time, min	Temp, deg C	Actual reading	Corrected reading	K	Rm	Eff. depth	Diameter mm	Percent finer
1.00	23.0	47.0	43.7	0.0138	48.0	8.4	0.0400	65.6
2.00	23.0	43.0	39.7	0.0138	44.0	9.1	0.0294	59.6
5.00	23.0	41.5	38.2	0.0138	42.5	9.3	0.0188	57.3
10.00	23.0	40.5	37.2	0.0138	41.5	9.5	0.0134	55.8
15.00	23.0	38.0	34.7	0.0138	39.0	9.9	0.0112	52.1
30.00	23.0	36.0	32.7	0.0138	37.0	10.2	0.0081	49.1
60.00	23.0	35.0	31.7	0.0138	36.0	10.4	0.0057	47.6
250.00	23.0	27.5	24.2	0.0138	28.5	11.6	0.0030	36.3
1440.00	24.0	20.0	17.0	0.0136	21.0	12.9	0.0013	25.5

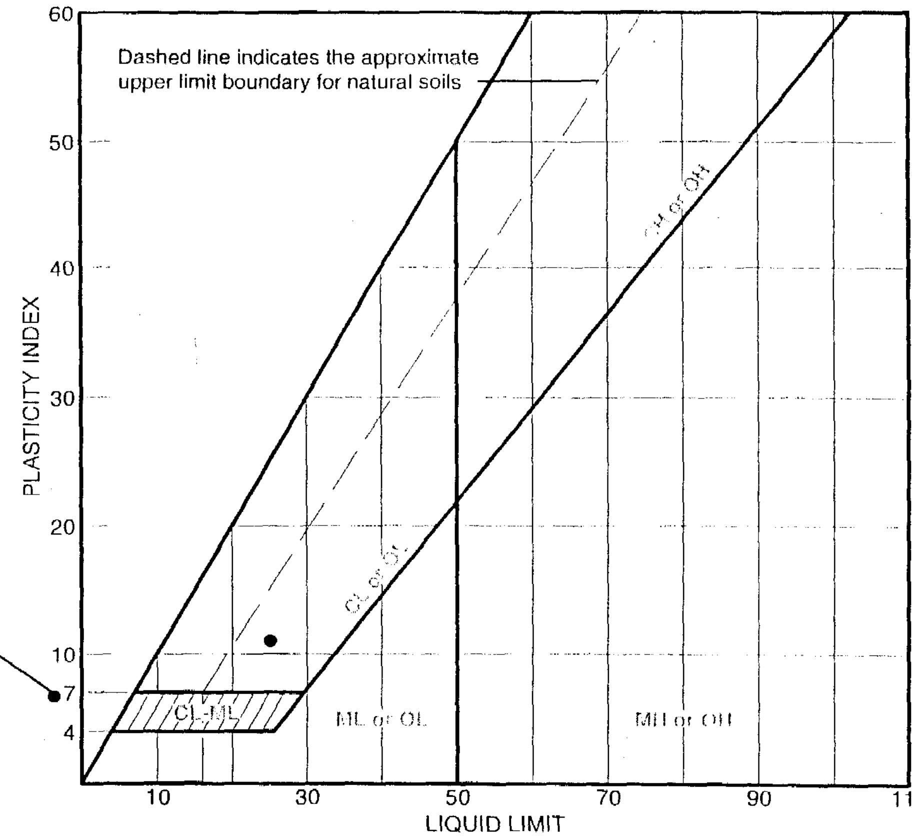
Fractional Components

Gravel/Sand based on #4
Sand/Fines based on #200
% + 3" = **% GRAVEL =**
% SAND = 32.6 **(% coarse =** 0.0 **% medium =** 1.0 **% fine =** 31.6)
% SILT = 21.4 **% CLAY =** 46.0

D_{85}= 0.14 D_{60}= 0.03 D_{50}= 0.01
D_{30}= 0.00

LIQUID AND PLASTIC LIMITS TEST REPORT





	SOURCE	SAMPLE #	DEPTH/ELEV.	DATE SAMPLED	USCS	MATERIAL DESCRIPTION	NM %	LL	PI
●	D&D TSF 06-26 soils		11.0' - 12.5'	6/10/03	CL	Sandy lean clay	14.9%	25.1	11.0

Client Project Project No. Plate	**INEEL MATERIALS LAB**	● Sample #1RA17604PR sampled May 14th, 2003. Borehole #4 south. Lab Log #027

LIQUID AND PLASTIC LIMIT TEST DATA

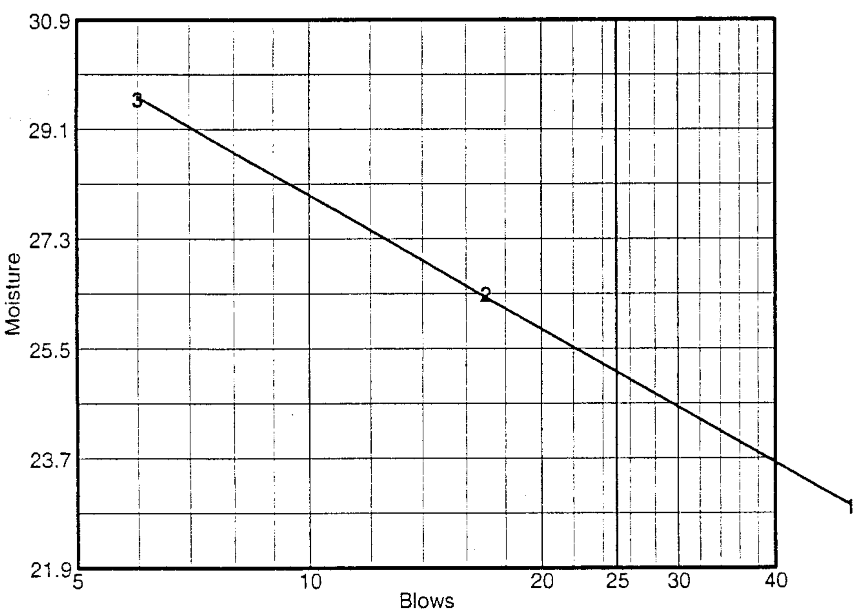
Client:
Project:
Project Number:

Sample Data

Source: D&D TSF 06-26 soils
Sample No.: 1RA17604PR
Elev. or Depth: 11.0' - 12.5'   **Sample Length (in./cm.):** LL #027
Location:
Description: Sandy lean clay
Date: 6/10/03   **Natural Moisture:** 14.9%
USCS Class.: CL   **AASHTO Class.:** A-6(5)
Testing Remarks: Sample #1RA17604PR sampled May 14th, 2003. Borehole #4 south. Lab Log #027

Liquid Limit Data

Run No.	1	2	3	4	5	6
Wet+Tare	31.89	27.93	27.78			
Dry+Tare	28.62	25.08	24.65			
Tare	14.35	14.29	14.09			
# Blows	50	17	6			
Moisture	22.9	26.4	29.6			

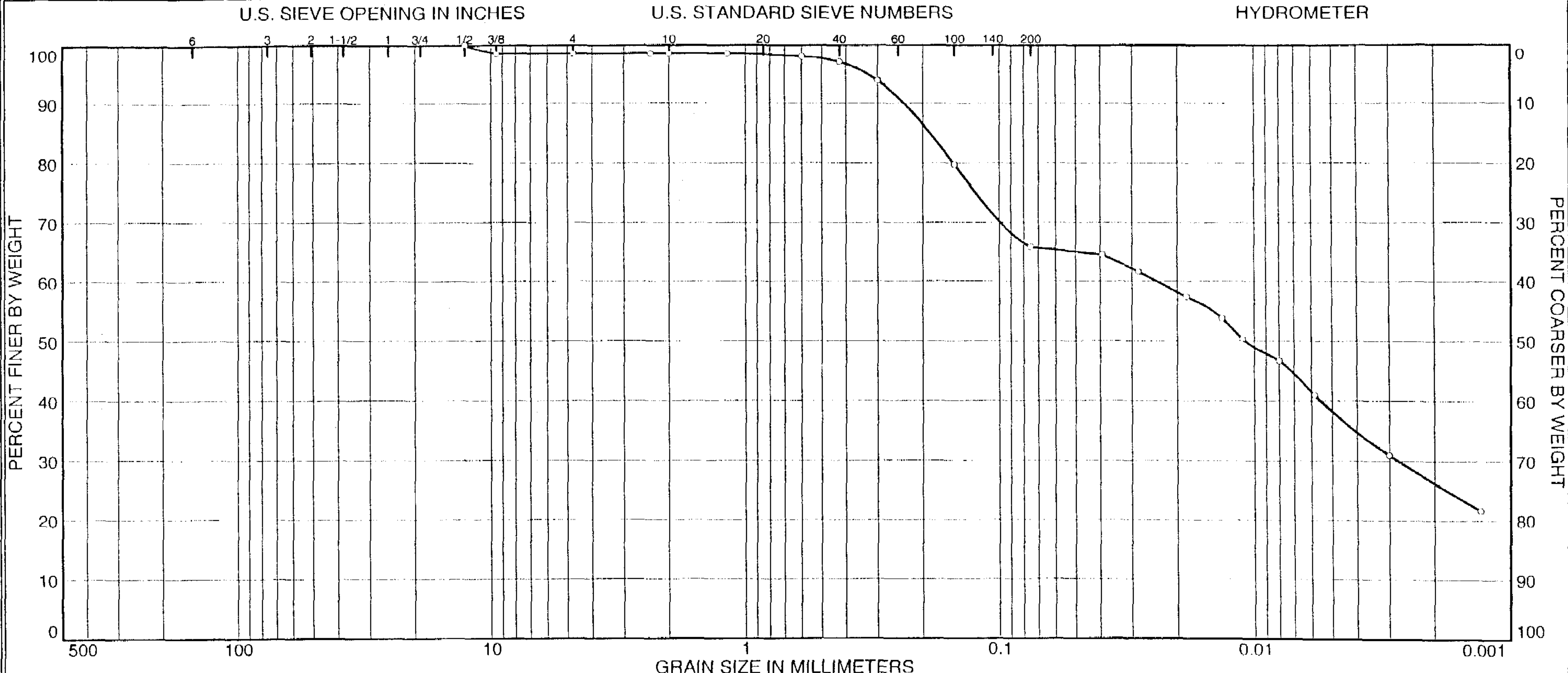


Liquid Limit= 25.1
Plastic Limit= 14.1
Plasticity Index= 11.0

Plastic Limit Data

Run No.	1	2	3	4
Wet+Tare	8.51	8.77		
Dry+Tare	8.00	8.21		
Tare	4.33	4.29		
isture	13.9	14.3		

PARTICLE SIZE DISTRIBUTION TEST REPORT



	% + 3"	% GRAVEL		% SAND			% FINES	
		COARSE	FINE	COARSE	MEDIUM	FINE	SILT	CLAY
○	0.0	0.0	1.6	0.0	1.4	31.2	27.6	38.2

	SOURCE	SAMPLE #	DEPTH/ELEV.	DATE SAMPLED	USCS	MATERIAL DESCRIPTION	NM %	LL	PL
○	D&D TSF 06-26 soils		14.0' - 16.5'	6/10/03	CL	Sandy lean clay	16.8%	23.4	14.3

Client

Project

Project No. Plate

INEEL MATERIALS LAB

○ Sample #1RA17605PR sampled May 14th, 2003. Borehole #5 South.

Lab Log #028

GRAIN SIZE DISTRIBUTION TEST DATA

Client:
Project:
Project Number:

Sample Data

Source: D&D TSF 06-26 soils
Sample No.: 1RA17605PR
Elev. or Depth: 14.0' - 16.5'　　**Sample Length (in./cm.):** LL #028
Location:
Description: Sandy lean clay
Date: 6/10/03　　**Natural Moisture:** 16.8%
Liquid Limit: 23.4　　**Plastic Limit:** 14.3　　**USCS Class.:** CL
Testing Remarks: Sample #1RA17605PR sampled May 14th, 2003. Borehole #5 South.

Lab Log #028

Mechanical Analysis Data

	Initial
Dry sample and tare=	388.30
Tare =	105.73
Dry sample weight =	282.57

Sieve tare method

Sieve	Weight retained	Sieve tare	Percent finer
1/2 inch	0.00	0.00	100.0
3/8 inch	4.42	0.00	98.4
4	0.00	0.00	98.4
# 8	0.00	0.00	98.4
# 10	0.07	0.00	98.4
# 16	0.10	0.00	98.4
# 30	1.37	0.00	97.9
# 40	2.42	0.00	97.0
# 50	9.20	0.00	93.8
# 100	39.92	0.00	79.7
# 200	39.11	0.00	65.8

Hydrometer Analysis Data

Separation sieve is #10
Percent -#10 based upon complete sample= 98.4
Weight of hydrometer sample: 73.19
Hygroscopic moisture correction:
- **Moist weight & tare =** 393.98
- **Dry weight & tare =** 388.30
- **Tare =** 105.73
- **Hygroscopic moisture=** 2.0 %

Calculated biased weight= 72.91
Automatic temperature correction
- **Composite correction at 20 deg C =** -4.0

Meniscus correction only= 1.0
Specific gravity of solids= 2.50
Specific gravity correction factor= 1.038
Hydrometer type: 152H

Effective depth L= 16.294964 - 0.164 x Rm

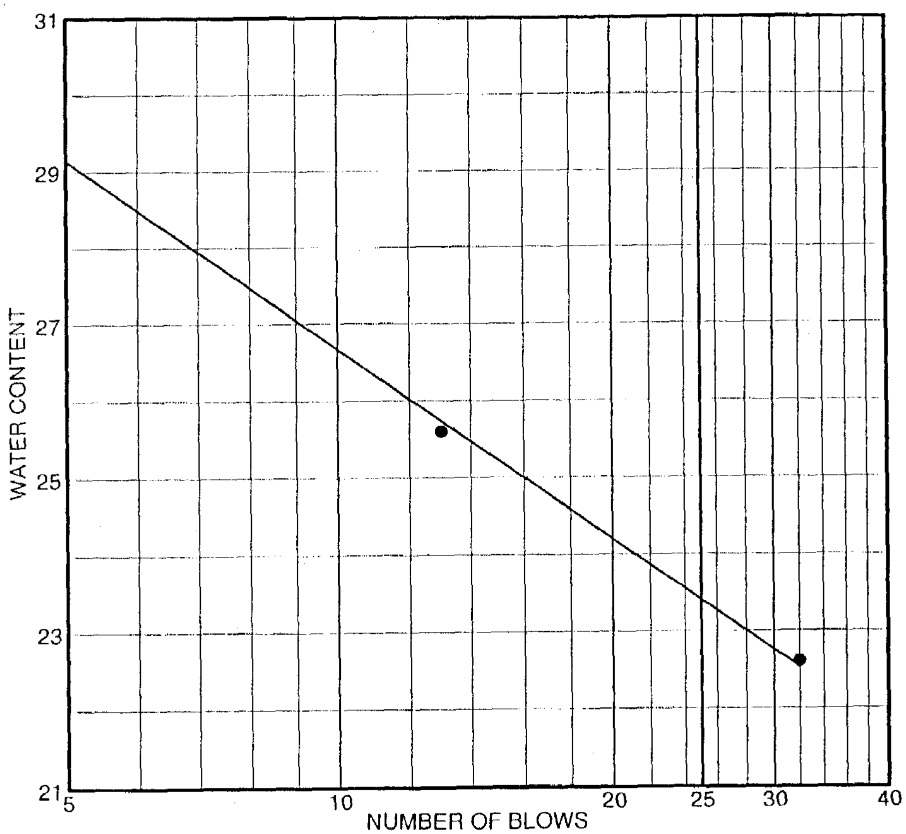
Elapsed time, min	Temp, deg C	Actual reading	Corrected reading	K	Rm	Eff. depth	Diameter mm	Percent finer
1.00	23.5	48.5	45.3	0.0137	49.5	8.2	0.0392	64.5
2.00	23.5	46.5	43.3	0.0137	47.5	8.5	0.0283	61.6
5.00	23.5	43.5	40.3	0.0137	44.5	9.0	0.0184	57.4
10.00	23.5	41.0	37.8	0.0137	42.0	9.4	0.0133	53.8
15.00	23.5	38.5	35.3	0.0137	39.5	9.8	0.0111	50.3
30.00	23.5	36.0	32.8	0.0137	37.0	10.2	0.0080	46.7
60.00	23.5	32.0	28.8	0.0137	33.0	10.9	0.0058	41.0
250.00	23.5	25.0	21.8	0.0137	26.0	12.0	0.0030	31.0
1440.00	23.0	18.5	15.2	0.0138	19.5	13.1	0.0013	21.6

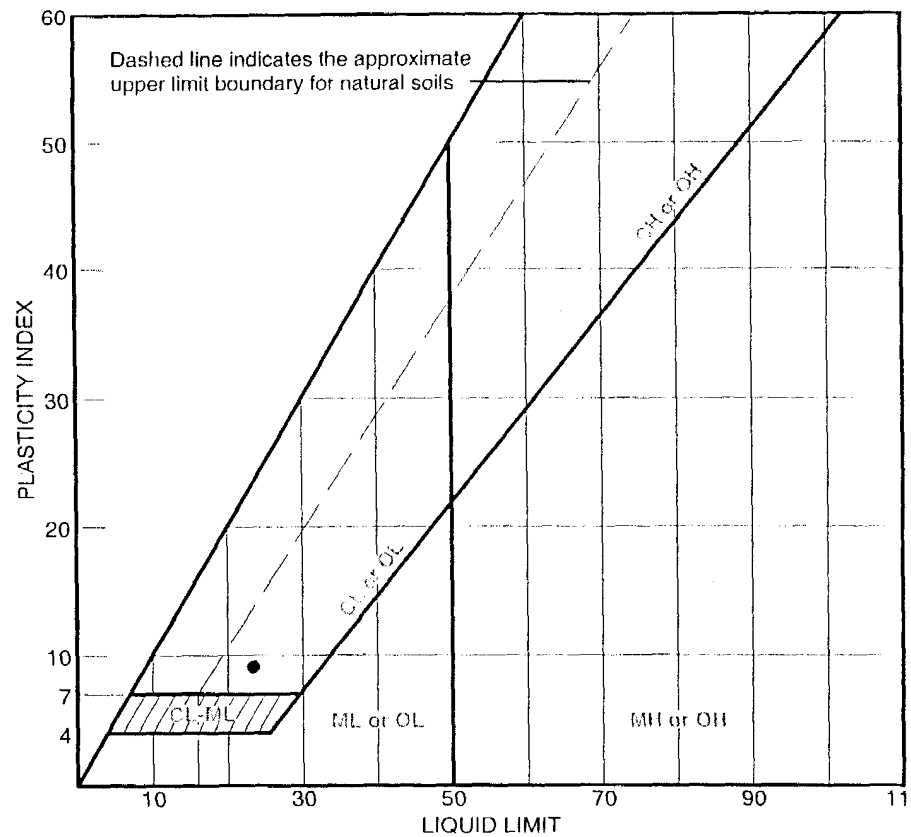
Fractional Components

Gravel/Sand based on #4
Sand/Fines based on #200
% + 3" = % GRAVEL = 1.6 (% coarse = % fine = 1.6)
% SAND = 32.6 (% coarse = 0.0 % medium = 1.4 % fine = 31.2)
% SILT = 27.6 % CLAY = 38.2

D_{85}= 0.19 D_{60}= 0.02 D_{50}= 0.01
D_{30}= 0.00

LIQUID AND PLASTIC LIMITS TEST REPORT





	SOURCE	SAMPLE #	DEPTH/ELEV.	DATE SAMPLED	USCS	MATERIAL DESCRIPTION	NM %	LL	PI
●	D&D TSF 06-26 soils		14.0' - 16.5'	6/10/03	CL	Sandy lean clay	16.8%	23.4	9.1

Client

Project

Project No.

Plate

INEEL MATERIALS LAB

● Sample #1RA17605PR sampled May 14th, 2003. Borehole #5 South.
Lab Log #028

LIQUID AND PLASTIC LIMIT TEST DATA

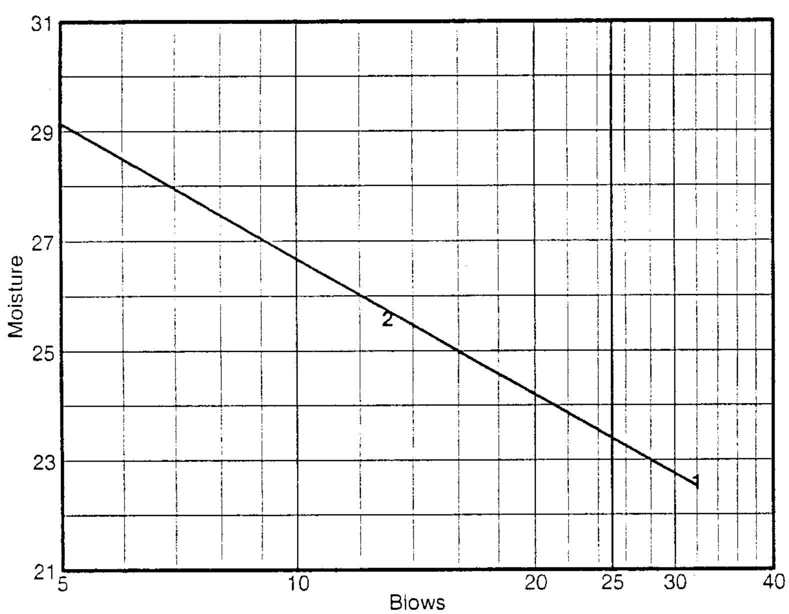
Client:
Project:
Project Number:

Sample Data

Source: D&D TSF 06-26 soils
Sample No.: 1RA17605PR
Elev. or Depth: 14.0' - 16.5' **Sample Length (in./cm.):** LL #028
Location:
Description: Sandy lean clay
Date: 6/10/03 **Natural Moisture:** 16.8%
USCS Class.: CL **AASHTO Class.:** A-4(3)
Testing Remarks: Sample #1RA17605PR sampled May 14th, 2003. Borehole #5 South. Lab Log #028

Liquid Limit Data

Run No.	1	2	3	4	5	6
Wet+Tare	31.66	30.50	30.91			
Dry+Tare	28.44	27.17	27.09			
Tare	14.20	14.14	14.34			
# Blows	32	13	4			
Moisture	22.6	25.6	30.0			

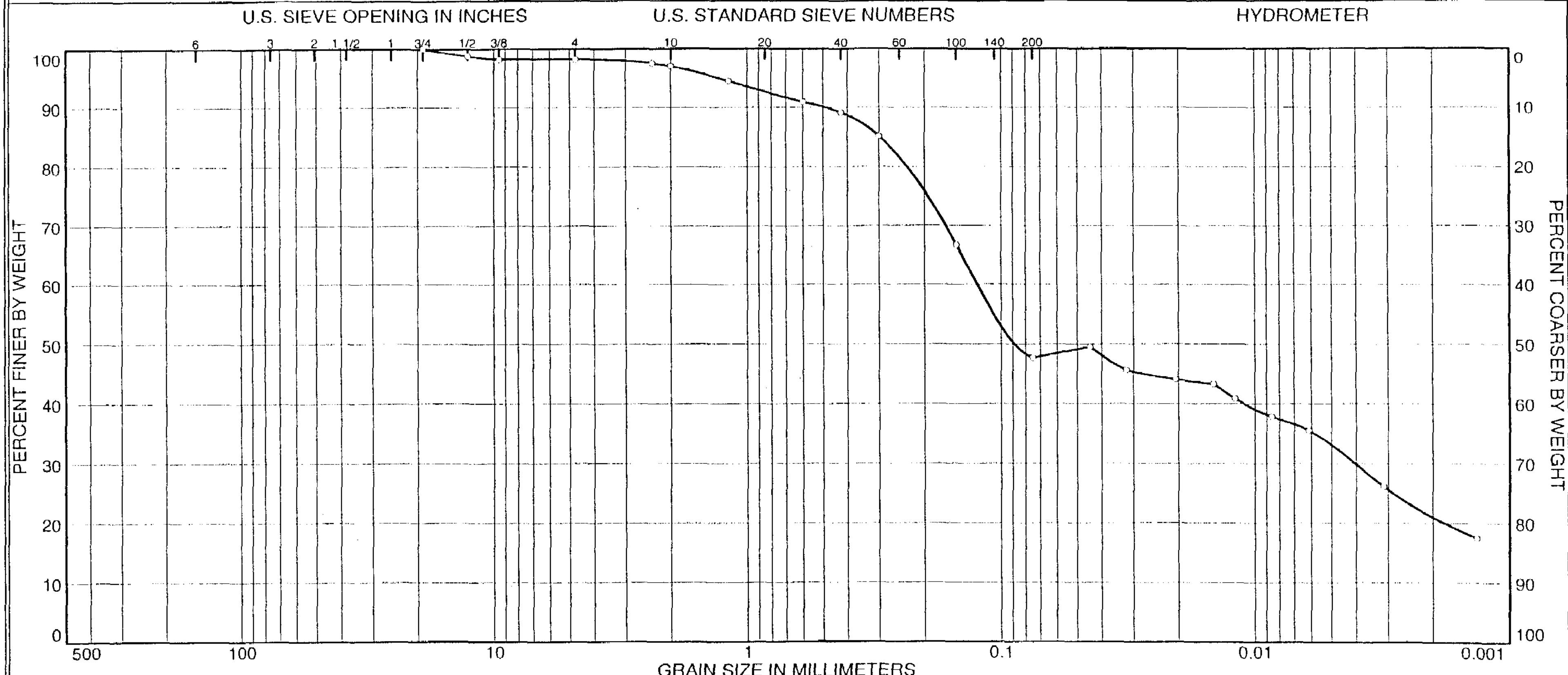


Liquid Limit= 23.4
Plastic Limit= 14.3
Plasticity Index= 9.1

Plastic Limit Data

Run No.	1	2	3	4
Wet+Tare	23.31	24.63		
Dry+Tare	22.48	23.75		
Tare	16.72	17.55		
isture	14.4	14.2		

PARTICLE SIZE DISTRIBUTION TEST REPORT



% + 3"	% GRAVEL		% SAND			% FINES	
	COARSE	FINE	COARSE	MEDIUM	FINE	SILT	CLAY
○ 0.0	0.0	1.9	1.2	7.8	41.4	14.6	33.1

SOURCE	SAMPLE #	DEPTH/ELEV.	DATE SAMPLED	USCS	MATERIAL DESCRIPTION	NM %	LL	PL
○ D&D TSF 06-26 soils		17.0' - 18.5'	6/10/03	SC	Clayey sand	12.3%	21.9	13.8

Client
Project
Project No. Plate

INEEL MATERIALS LAB

○ Sample #1RA17606PR sampled May 14th, 2003. Borehole #6 South.

Lab Log #029

GRAIN SIZE DISTRIBUTION TEST DATA

Client:
Project:
Project Number:

Sample Data

Source: D&D TSF 06-26 soils
Sample No.: 1RA17606PR
Elev. or Depth: 17.0' - 18.5' **Sample Length (in./cm.):** LL #029
Location:
Description: Clayey sand
Date: 6/10/03 **Natural Moisture:** 12.3%
Liquid Limit: 21.9 **Plastic Limit:** 13.8 **USCS Class.:** SC
Testing Remarks: Sample #1RA17606PR sampled May 14th, 2003. Borehole #6 South.

Lab Log #029

Mechanical Analysis Data

		Initial
Dry sample and tare	=	459.96
Tare	=	106.10
Dry sample weight	=	353.86

Sieve tare method

Sieve	Weight retained	Sieve tare	Percent finer
3/4 inch	0.00	0.00	100.0
1/2 inch	4.58	0.00	98.7
3/8 inch	2.31	0.00	98.1
# 4	0.00	0.00	98.1
# 8	2.39	0.00	97.4
# 10	1.72	0.00	96.9
# 16	8.90	0.00	94.4
# 30	12.26	0.00	90.9
# 40	6.48	0.00	89.1
# 50	14.09	0.00	85.1
# 100	64.95	0.00	66.7
# 200	67.29	0.00	47.7

Hydrometer Analysis Data

Separation sieve is #10
Percent -#10 based upon complete sample= 96.9
Weight of hydrometer sample: 64.95
Hygroscopic moisture correction:
  Moist weight & tare = 463.62
  Dry weight & tare = 459.96
  Tare = 106.10
  Hygroscopic moisture= 1.0 %
Calculated biased weight= 66.34
Automatic temperature correction
  Composite correction at 20 deg C = -4.0

Meniscus correction only= 1.0
Specific gravity of solids= 2.50
Specific gravity correction factor= 1.038

Hydrometer type: 152H
Effective depth L= 16.294964 - 0.164 x Rm

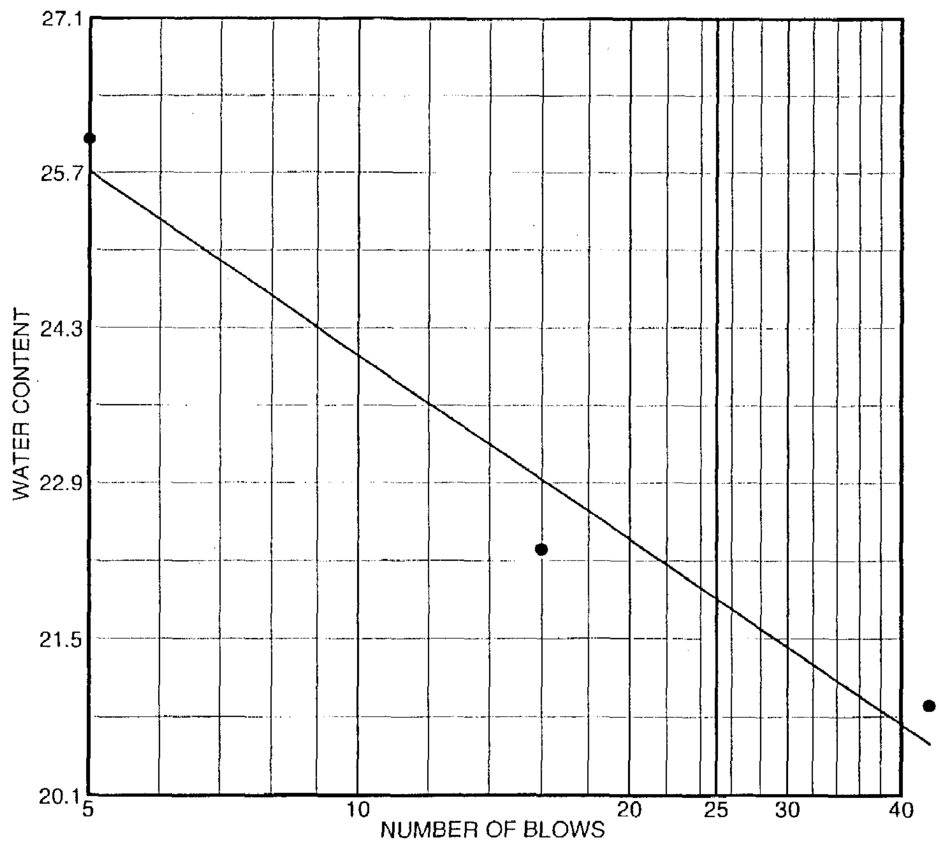
Elapsed time, min	Temp, deg C	Actual reading	Corrected reading	K	Rm	Eff. depth	Diameter mm	Percent finer
1.00	23.0	35.0	31.7	0.0138	36.0	10.4	0.0445	49.5
2.00	23.0	32.5	29.2	0.0138	33.5	10.8	0.0321	45.6
5.00	23.0	31.5	28.2	0.0138	32.5	11.0	0.0204	44.1
10.00	23.0	31.0	27.7	0.0138	32.0	11.0	0.0145	43.3
15.00	23.0	29.5	26.2	0.0138	30.5	11.3	0.0120	40.9
30.00	23.0	27.5	24.2	0.0138	28.5	11.6	0.0086	37.8
60.00	23.0	26.0	22.7	0.0138	27.0	11.9	0.0061	35.5
250.00	23.0	20.0	16.7	0.0138	21.0	12.9	0.0031	26.1
1440.00	23.0	14.5	11.2	0.0138	15.5	13.8	0.0013	17.5

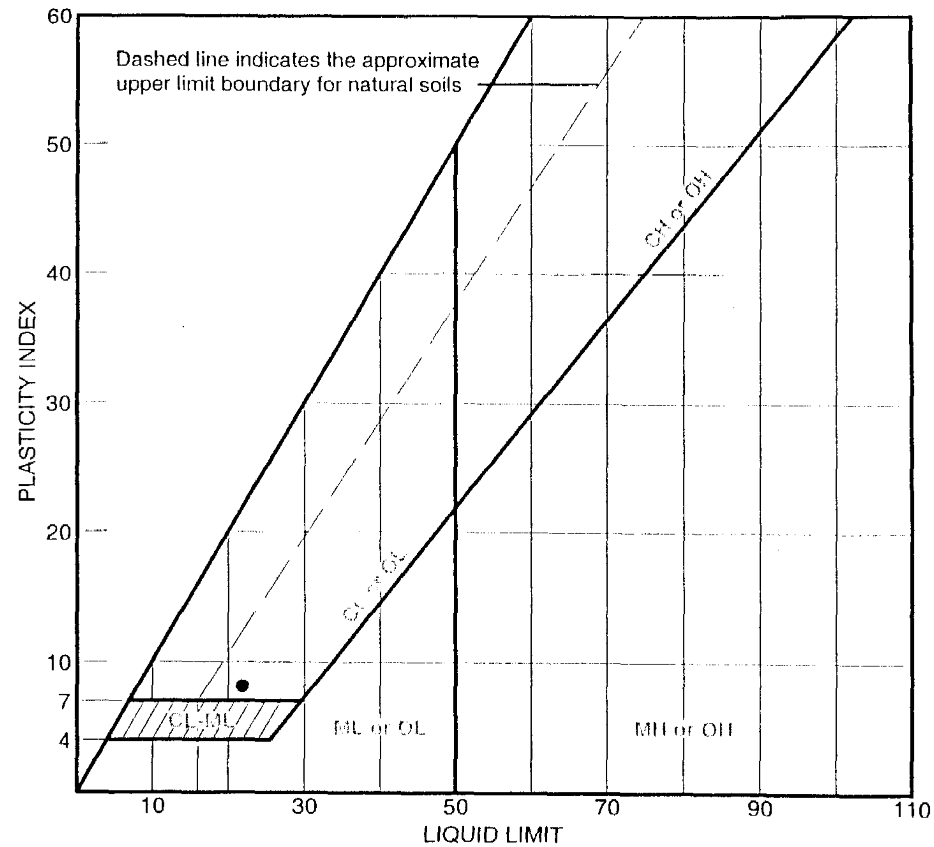
Fractional Components

Gravel/Sand based on #4
Sand/Fines based on #200
% + 3" = % GRAVEL = 1.9 (% coarse = % fine = 1.9)
% SAND = 50.4 (% coarse = 1.2 % medium = 7.8 % fine = 41.4)
% SILT = 14.6 % CLAY = 33.1

D_{85}= 0.30 D_{60}= 0.12 D_{50}= 0.09
D_{30}= 0.00

LIQUID AND PLASTIC LIMITS TEST REPORT





	SOURCE	SAMPLE #	DEPTH/ELEV.	DATE SAMPLED	USCS	MATERIAL DESCRIPTION	NM %	LL	PI
●	D&D TSF 06-26 soils		17.0' - 18.5'	6/10/03	SC	Clayey sand	12.3%	21.9	8.1

Client Project	**INEEL MATERIALS LAB**	● Sample #1RA17606PR sampled May 14th, 2003. Borehole #6 South. Lab Log #029
Project No. Plate		

LIQUID AND PLASTIC LIMIT TEST DATA

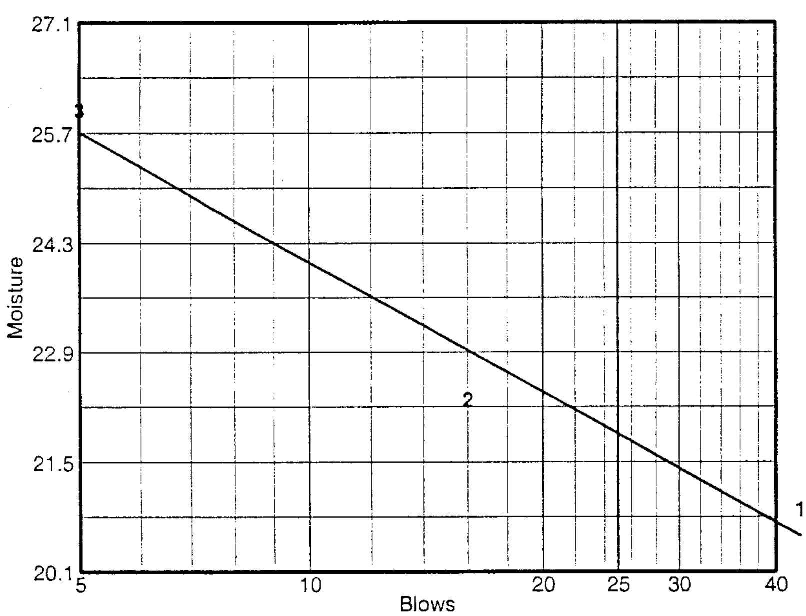
Client:
Project:
Project Number:

Sample Data

Source: D&D TSF 06-26 soils
Sample No.: 1RA17606PR
Elev. or Depth: 17.0' - 18.5' **Sample Length (in./cm.):** LL #029
Location:
Description: Clayey sand
Date: 6/10/03 **Natural Moisture:** 12.3%
USCS Class.: SC **AASHTO Class.:** A-4(1)
Testing Remarks: Sample #1RA17606PR sampled May 14th, 2003. Borehole #6 South. Lab Log #029

Liquid Limit Data

Run No.	1	2	3	4	5	6
Wet+Tare	26.39	28.00	25.77			
Dry+Tare	23.75	24.92	22.74			
Tare	11.12	11.10	11.09			
# Blows	43	16	5			
Moisture	20.9	22.3	26.0			

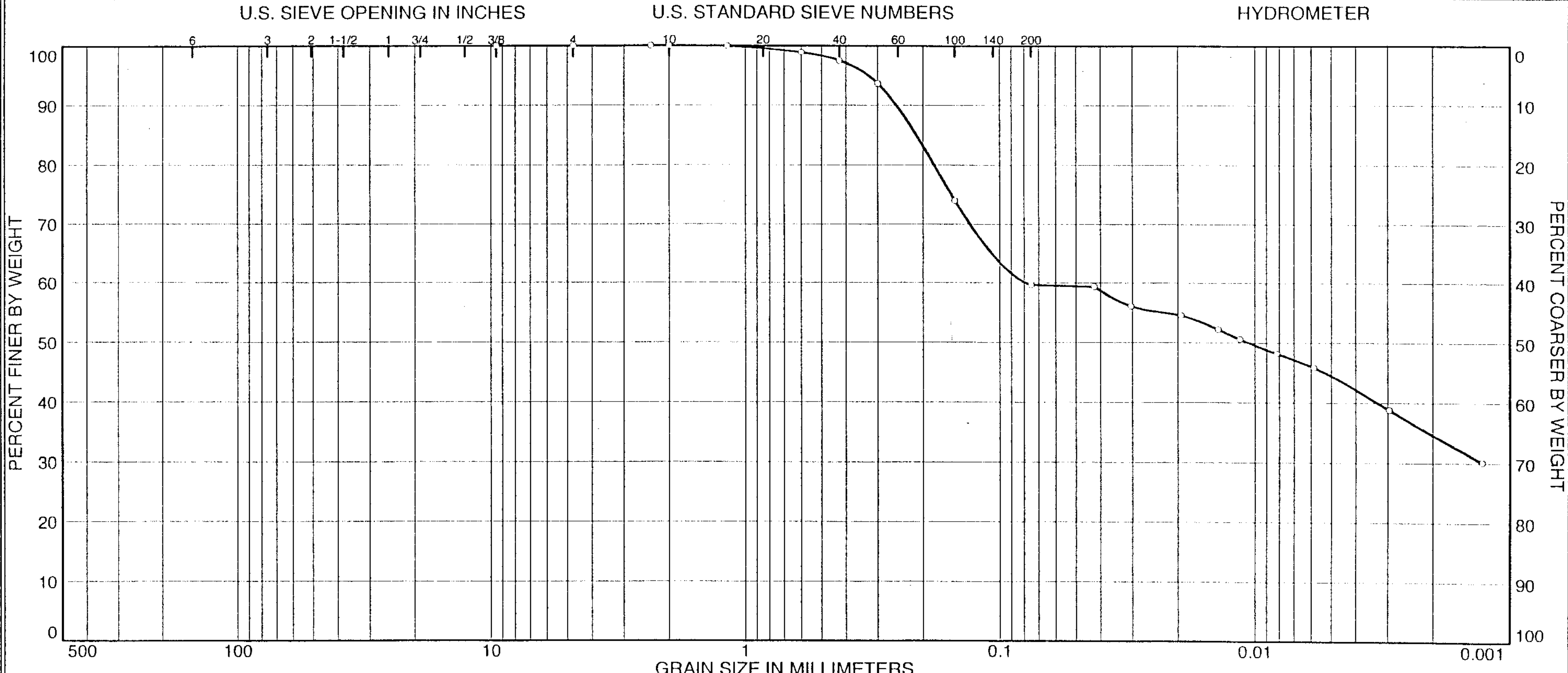


Liquid Limit= 21.9
Plastic Limit= 13.8
Plasticity Index= 8.1

Plastic Limit Data

Run No.	1	2	3	4
Wet+Tare	10.13	10.31		
Dry+Tare	9.44	9.58		
Tare	4.33	4.36		
Moisture	13.5	14.0		

PARTICLE SIZE DISTRIBUTION TEST REPORT



	% + 3"	% GRAVEL		% SAND			% FINES	
		COARSE	FINE	COARSE	MEDIUM	FINE	SILT	CLAY
○	0.0	0.0	0.0	0.0	2.6	37.8	15.1	44.5

	SOURCE	SAMPLE #	DEPTH/ELEV.	DATE SAMPLED	USCS	MATERIAL DESCRIPTION	NM %	LL	PL
○	D&D TSF 06-26 soils		20.0' - 21.5'	6/10/03	CL	Sandy lean clay	18.3%	26.7	14.2

Client
Project

Project No. Plate

INEEL MATERIALS LAB

○ Sample #1RA17607PR sampled May 14th, 2003. Borehole #7 South.

LAb Log #030

GRAIN SIZE DISTRIBUTION TEST DATA

Client:
Project:
Project Number:

Sample Data

Source: D&D TSF 06-26 soils
Sample No.: 1RA17607PR
Elev. or Depth: 20.0' - 21.5' **Sample Length (in./cm.):** LL #030
Location:
Description: Sandy lean clay
Date: 6/10/03 **Natural Moisture:** 18.3%
Liquid Limit: 26.7 **Plastic Limit:** 14.2 **USCS Class.:** CL
Testing Remarks: Sample #1RA17607PR sampled May 14th, 2003. Borehole #7 South.

LAb Log #030

Mechanical Analysis Data

	Initial
Dry sample and tare=	381.83
Tare =	106.14
Dry sample weight =	275.69

Sieve tare method

Sieve	Weight retained	Sieve tare	Percent finer
3/8 inch	0.00	0.00	100.0
# 4	0.00	0.00	100.0
# 8	0.00	0.00	100.0
# 10	0.00	0.00	100.0
# 16	0.21	0.00	99.9
# 30	3.06	0.00	98.8
# 40	3.78	0.00	97.4
# 50	10.65	0.00	93.6
# 100	53.91	0.00	74.0
# 200	39.85	0.00	59.6

Hydrometer Analysis Data

Separation sieve is #10
Percent -#10 based upon complete sample= 100.0
Weight of hydrometer sample: 67.33
Hygroscopic moisture correction:
  Moist weight & tare = 387.47
  Dry weight & tare = 381.83
  Tare = 106.14
  Hygroscopic moisture= 2.0 %
Calculated biased weight= 65.98
Automatic temperature correction
  Composite correction at 20 deg C = -4.0

Meniscus correction only= 1.0
Specific gravity of solids= 2.50
Specific gravity correction factor= 1.038
Hydrometer type: 152H
  Effective depth L= 16.294964 - 0.164 x Rm

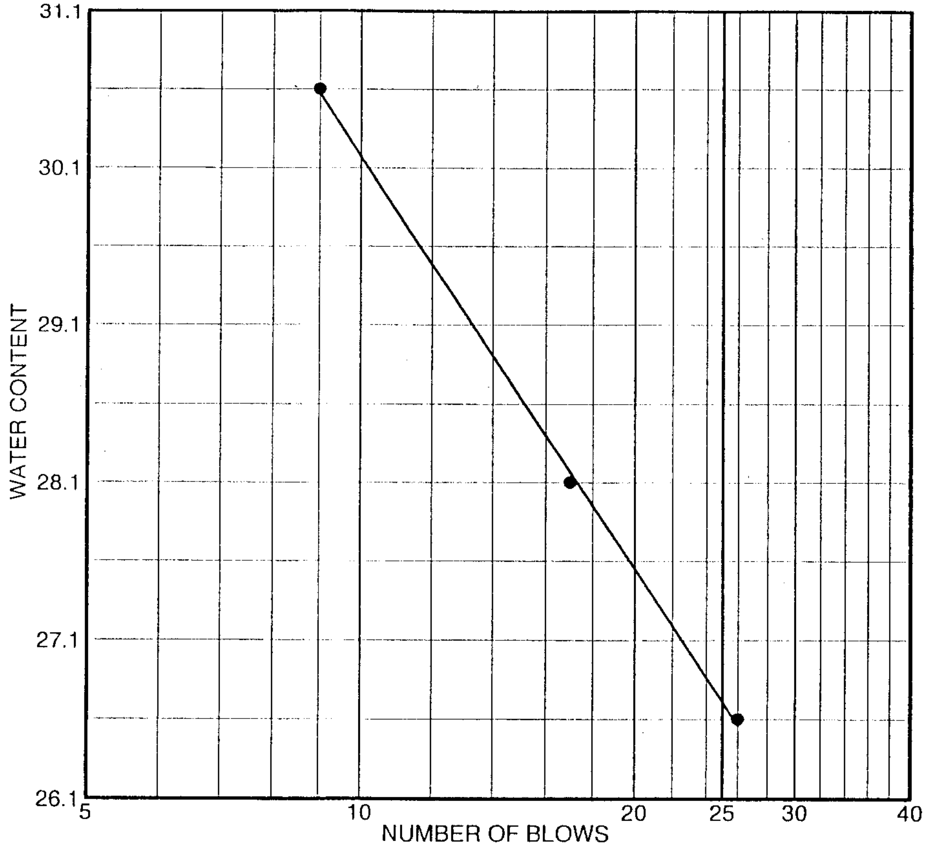
Elapsed time, min	Temp, deg C	Actual reading	Corrected reading	K	Rm	Eff. depth	Diameter mm	Percent finer
1.00	23.0	41.0	37.7	0.0138	42.0	9.4	0.0423	59.3
2.00	23.0	39.0	35.7	0.0138	40.0	9.7	0.0304	56.1
5.00	23.0	38.0	34.7	0.0138	39.0	9.9	0.0194	54.5
10.00	23.0	36.5	33.2	0.0138	37.5	10.1	0.0139	52.2
15.00	23.0	35.5	32.2	0.0138	36.5	10.3	0.0114	50.6
30.00	23.0	34.0	30.7	0.0138	35.0	10.6	0.0082	48.2
60.00	23.0	32.5	29.2	0.0138	33.5	10.8	0.0059	45.9
250.00	23.0	28.0	24.7	0.0138	29.0	11.5	0.0030	38.8
1440.00	23.0	22.5	19.2	0.0138	23.5	12.4	0.0013	30.1

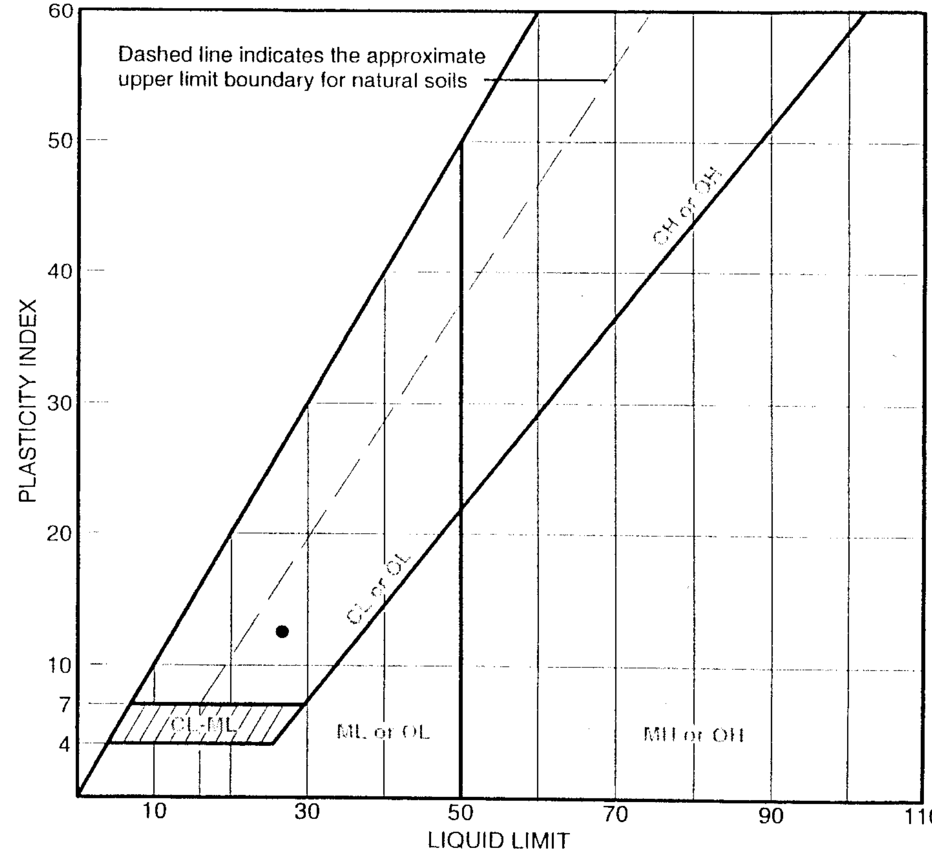
Fractional Components

Gravel/Sand based on #4
Sand/Fines based on #200
% + 3" = **% GRAVEL =**
% SAND = 40.4 **(% coarse =** 0.0 **% medium =** 2.6 **% fine =** 37.8)
% SILT = 15.1 **% CLAY =** 44.5

D_{85}= 0.21 D_{60}= 0.08 D_{50}= 0.01

LIQUID AND PLASTIC LIMITS TEST REPORT





	SOURCE	SAMPLE #	DEPTH/ELEV.	DATE SAMPLED	USCS	MATERIAL DESCRIPTION	NM %	LL	PI
●	D&D TSF 06-26 soils		20.0' - 21.5'	6/10/03	CL	Sandy lean clay	18.3%	26.7	12.5

Client Project Project No. / Plate	**INEEL MATERIALS LAB**	● Sample #1RA17607PR sampled May 14th, 2003. Borehole #7 South. Lab Log #030

LIQUID AND PLASTIC LIMIT TEST DATA

Client:
Project:
Project Number:

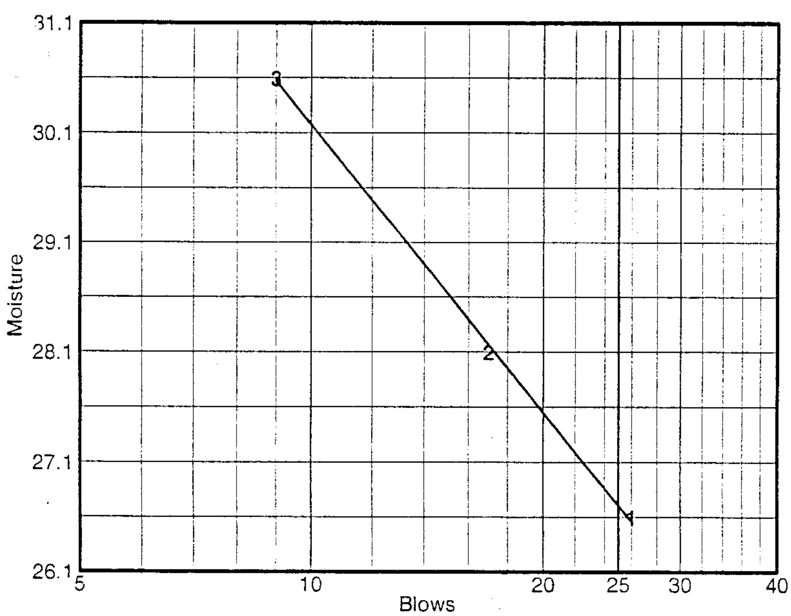
Sample Data

Source: D&D TSF 06-26 soils
Sample No.: 1RA17607PR
Elev. or Depth: 20.0' - 21.5'   **Sample Length (in./cm.):** LL #030
Location:
Description: Sandy lean clay
Date: 6/10/03   **Natural Moisture:** 18.3%
USCS Class.: CL   **AASHTO Class.:** A-6(5)
Testing Remarks: Sample #1RA17607PR sampled May 14th, 2003. Borehole #7 South.

Lab Log #030

Liquid Limit Data

Run No.	1	2	3	4	5	6
Wet+Tare	25.40	25.20	26.66			
Dry+Tare	22.40	22.11	23.02			
Tare	11.13	11.13	11.14			
# Blows	26	17	9			
Moisture	26.6	28.1	30.6			

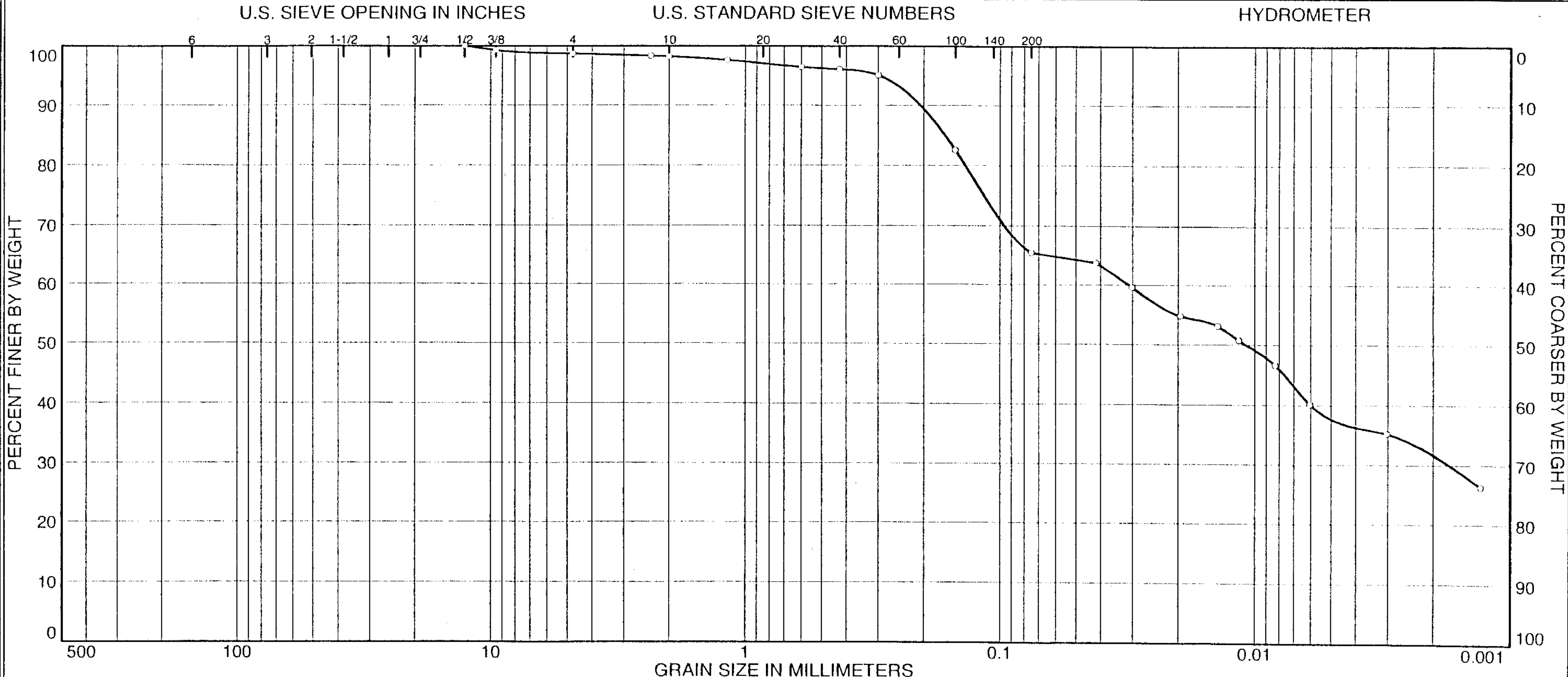


Liquid Limit= 26.7
Plastic Limit= 14.2
Plasticity Index= 12.5

Plastic Limit Data

Run No.	1	2	3	4
Wet+Tare	9.50	8.54		
Dry+Tare	8.85	8.02		
Tare	4.32	4.34		
Moisture	14.3	14.1		

PARTICLE SIZE DISTRIBUTION TEST REPORT



% + 3"	% GRAVEL		% SAND			% FINES	
	COARSE	FINE	COARSE	MEDIUM	FINE	SILT	CLAY
○ 0.0	0.0	1.4	0.5	2.1	30.6	27.8	37.6

SOURCE	SAMPLE #	DEPTH/ELEV.	DATE SAMPLED	USCS	MATERIAL DESCRIPTION	NM %	LL	PL
○ D&D TSF 06-26 soils		23.0' - 24.5'	6/10/03	CL	Sandy lean clay	20.5%	28.9	15.6

Client

Project

Project No. Plate

INEEL MATERIALS LAB

○ Sample #1RA17608PR sampled May 14th, 2003. Borehole #8 South.

Lab Log #031

GRAIN SIZE DISTRIBUTION TEST DATA

Client:
Project:
Project Number:

Sample Data

Source: D&D TSF 06-26 soils
Sample No.: 1RA17608PR
Elev. or Depth: 23.0' - 24.5' **Sample Length (in./cm.):** LL #031
Location:
Description: Sandy lean clay
Date: 6/10/03 **Natural Moisture:** 20.5%
Liquid Limit: 28.9 **Plastic Limit:** 15.6 **USCS Class.:** CL
Testing Remarks: Sample #1RA17608PR sampled May 14th, 2003. Borehole #8 South.

Lab Log #031

Mechanical Analysis Data

		Initial
Dry sample and tare	=	361.02
Tare	=	105.69
Dry sample weight	=	255.33

Sieve tare method

Sieve	Weight retained	Sieve tare	Percent finer
1/2 inch	0.00	0.00	100.0
3/8 inch	2.00	0.00	99.2
4	1.69	0.00	98.6
# 8	0.85	0.00	98.2
# 10	0.26	0.00	98.1
# 16	1.58	0.00	97.5
# 30	2.93	0.00	96.4
# 40	0.99	0.00	96.0
# 50	2.42	0.00	95.0
# 100	32.05	0.00	82.5
# 200	43.48	0.00	65.4

Hydrometer Analysis Data

Separation sieve is #10
Percent -#10 based upon complete sample= 98.1
Weight of hydrometer sample: 64.46
Hygroscopic moisture correction:
  Moist weight & tare = 368.59
  Dry weight & tare = 361.02
  Tare = 105.69
  Hygroscopic moisture= 3.0 %
Calculated biased weight= 63.82
Automatic temperature correction
  Composite correction at 20 deg C = -4.0

Meniscus correction only= 1.0
Specific gravity of solids= 2.50
Specific gravity correction factor= 1.038
Hydrometer type: 152H

Effective depth L= 16.294964 - 0.164 x Rm

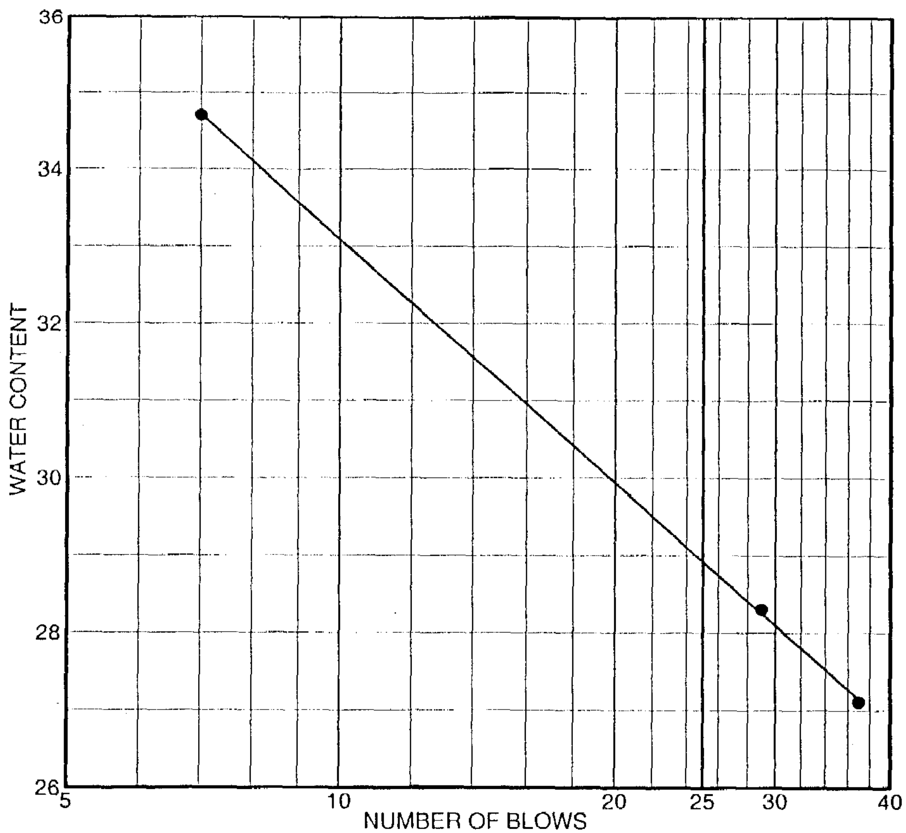
Elapsed time, min	Temp, deg C	Actual reading	Corrected reading	K	Rm	Eff. depth	Diameter mm	Percent finer
1.00	23.0	42.5	39.2	0.0138	43.5	9.2	0.0418	63.7
2.00	23.0	40.0	36.7	0.0138	41.0	9.6	0.0302	59.6
5.00	23.0	37.0	33.7	0.0138	38.0	10.1	0.0196	54.8
10.00	23.0	36.0	32.7	0.0138	37.0	10.2	0.0140	53.1
15.00	23.0	34.5	31.2	0.0138	35.5	10.5	0.0115	50.7
30.00	23.0	32.0	28.7	0.0138	33.0	10.9	0.0083	46.6
60.00	23.0	28.0	24.7	0.0138	29.0	11.5	0.0060	40.1
250.00	23.0	25.0	21.7	0.0138	26.0	12.0	0.0030	35.2
1440.00	23.0	19.5	16.2	0.0138	20.5	12.9	0.0013	26.3

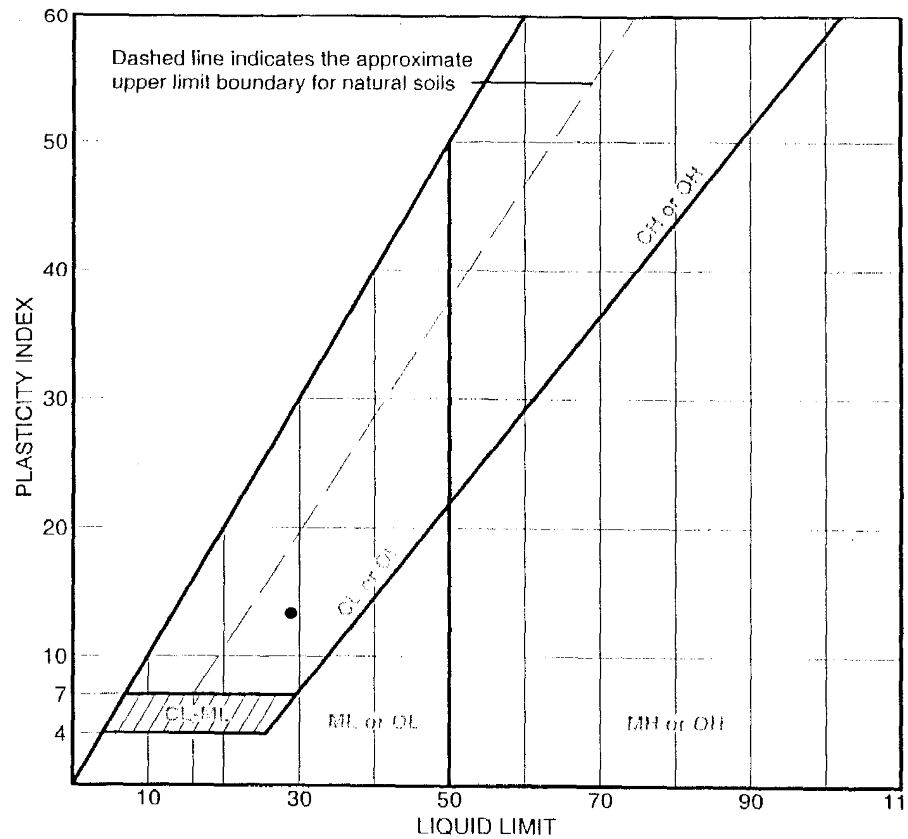
Fractional Components

Gravel/Sand based on #4
Sand/Fines based on #200
% + 3" = **% GRAVEL =** 1.4 **(% coarse =** **% fine =** 1.4)
% SAND = 33.2 **(% coarse =** 0.5 **% medium =** 2.1 **% fine =** 30.6)
% SILT = 27.8 **% CLAY =** 37.6

D_{85}= 0.16 D_{60}= 0.03 D_{50}= 0.01
D_{30}= 0.00

LIQUID AND PLASTIC LIMITS TEST REPORT





	SOURCE	SAMPLE #	DEPTH/ELEV.	DATE SAMPLED	USCS	MATERIAL DESCRIPTION	NM %	LL	PI
●	D&D TSF 06-26 soils		23.0' - 24.5'	6/10/03	CL	Sandy lean clay	20.5%	28.9	13.3

Client Project Project No. Plate	**INEEL MATERIALS LAB**	● Sample #1RA17608PR sampled May 14th, 2003. Borehole #8 South. Lab Log #031

LIQUID AND PLASTIC LIMIT TEST DATA

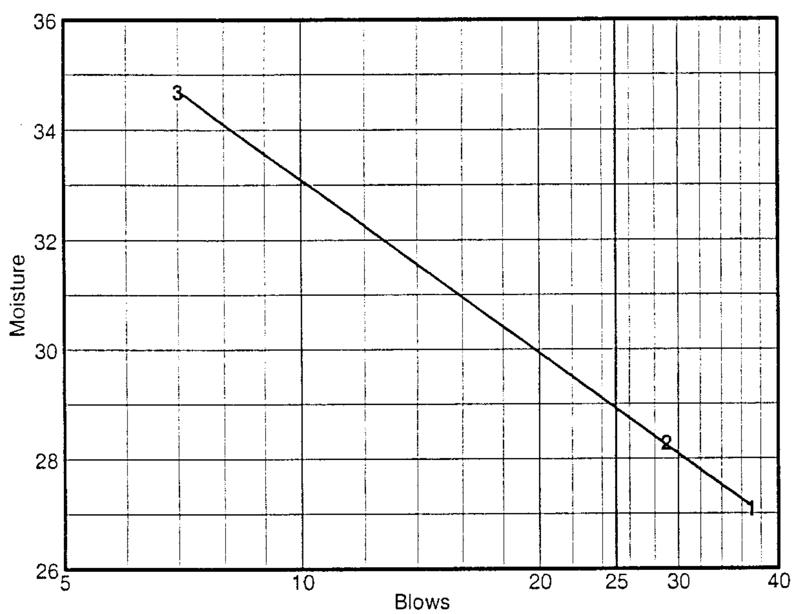
Client:
Project:
Project Number:

Sample Data

Source: D&D TSF 06-26 soils
Sample No.: 1RA17608PR
Elev. or Depth: 23.0' - 24.5' **Sample Length (in./cm.):** LL #031
Location:
Description: Sandy lean clay
Date: 6/10/03 **Natural Moisture:** 20.5%
USCS Class.: CL **AASHTO Class.:** A-6(6)
Testing Remarks: Sample #1RA17608PR sampled May 14th, 2003. Borehole #8 South. Lab Log #031

Liquid Limit Data

Run No.	1	2	3	4	5	6
Wet+Tare	62.16	66.66	65.21			
Dry+Tare	58.45	62.63	60.67			
Tare	44.77	48.37	47.58			
# Blows	37	29	7			
Moisture	27.1	28.3	34.7			

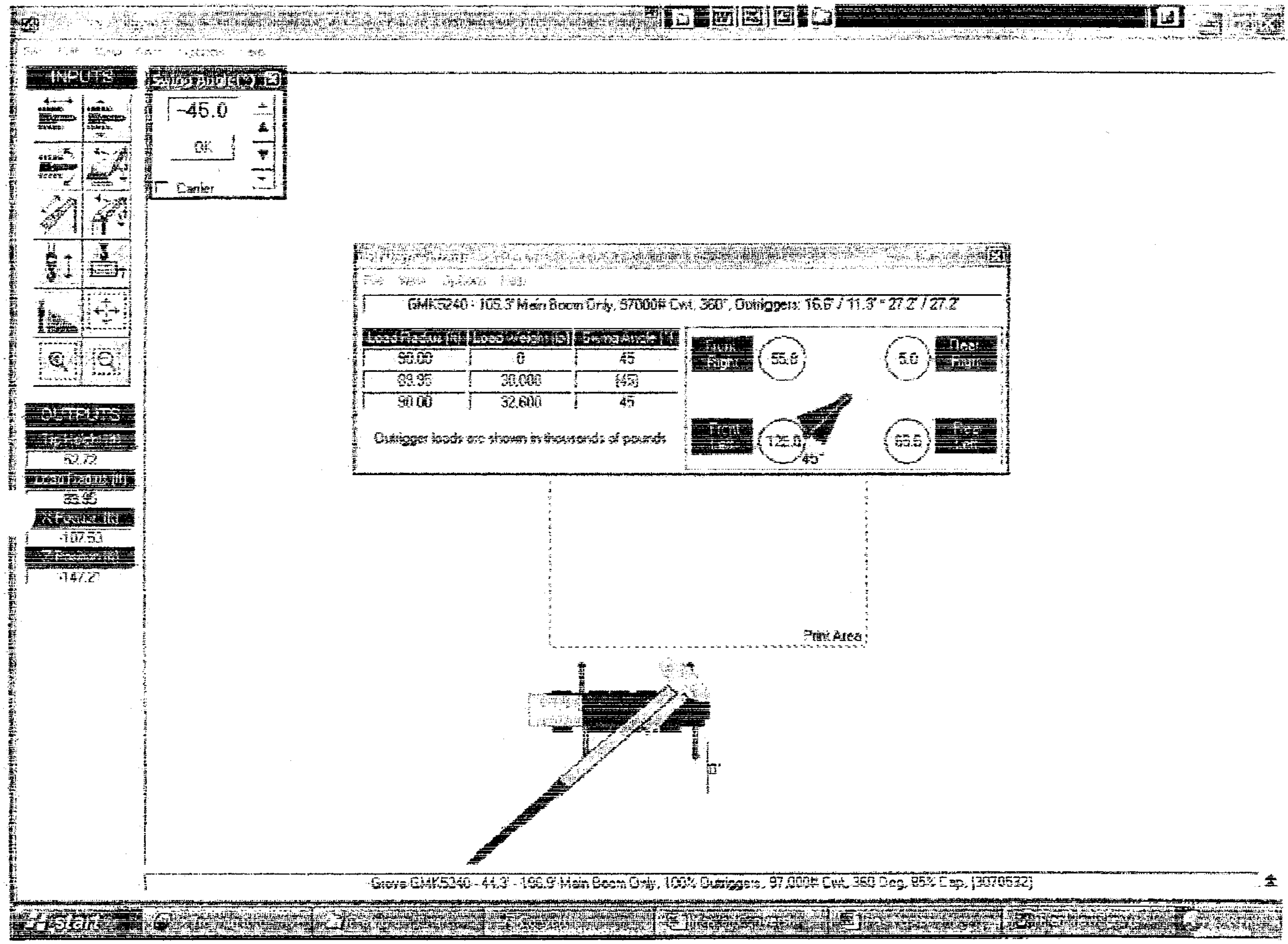


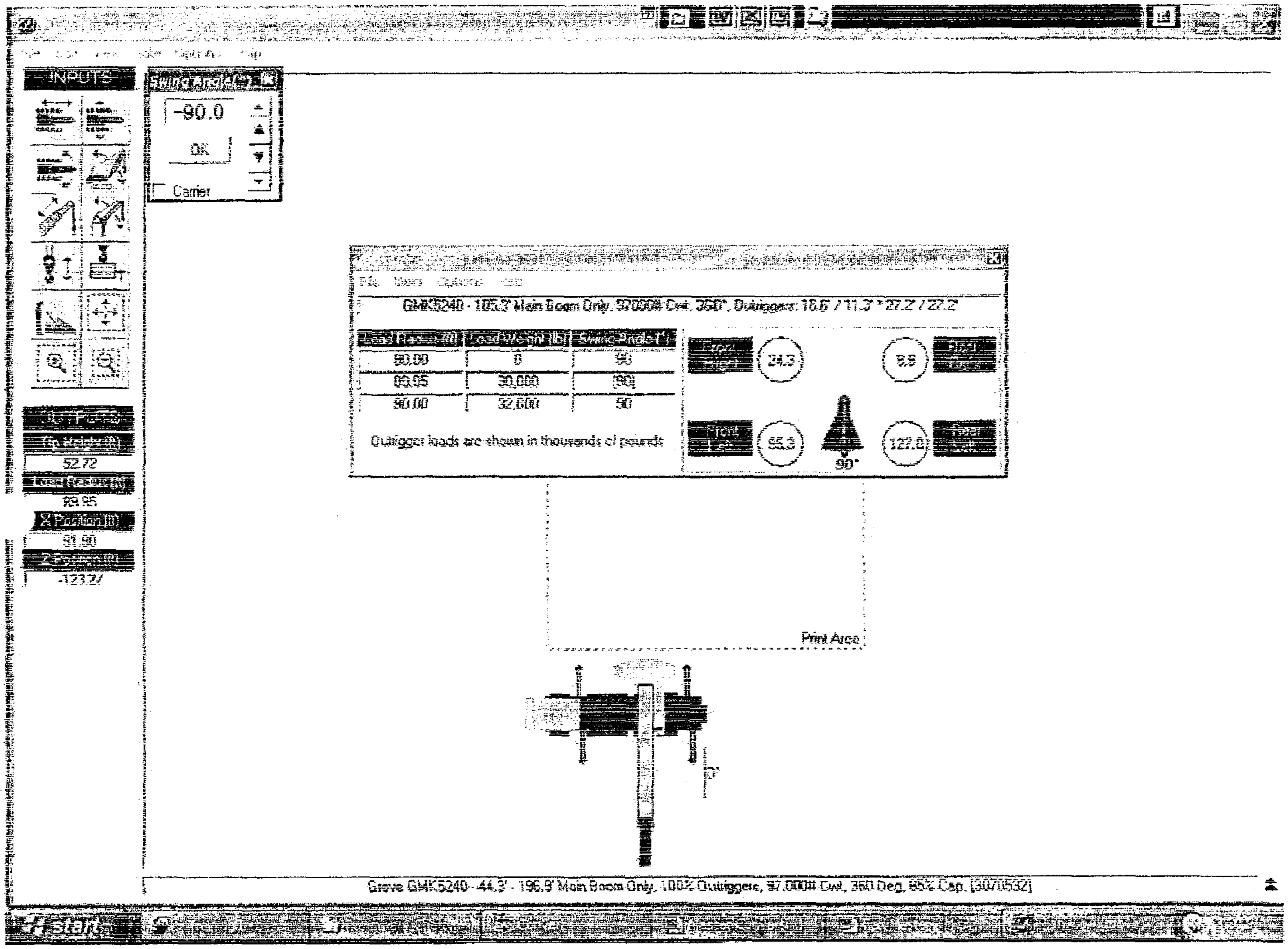
Liquid Limit= 28.9
Plastic Limit= 15.6
Plasticity Index= 13.3

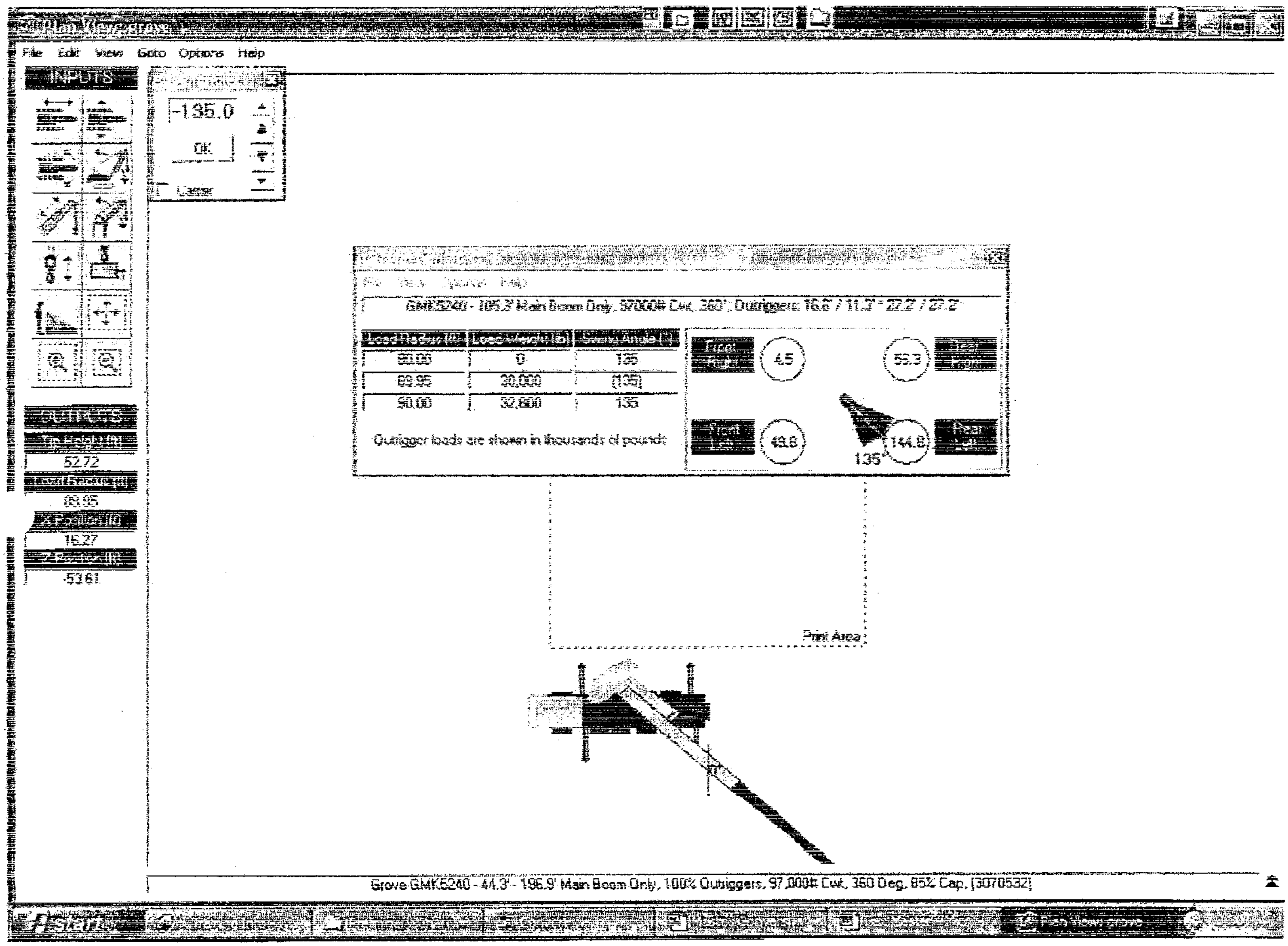
Plastic Limit Data

Run No.	1	2	3	4
Wet+Tare	21.16	22.25		
Dry+Tare	20.51	21.60		
Tare	16.38	17.40		
isture	15.7	15.5		

Appendix D: Crane Surcharge Loading

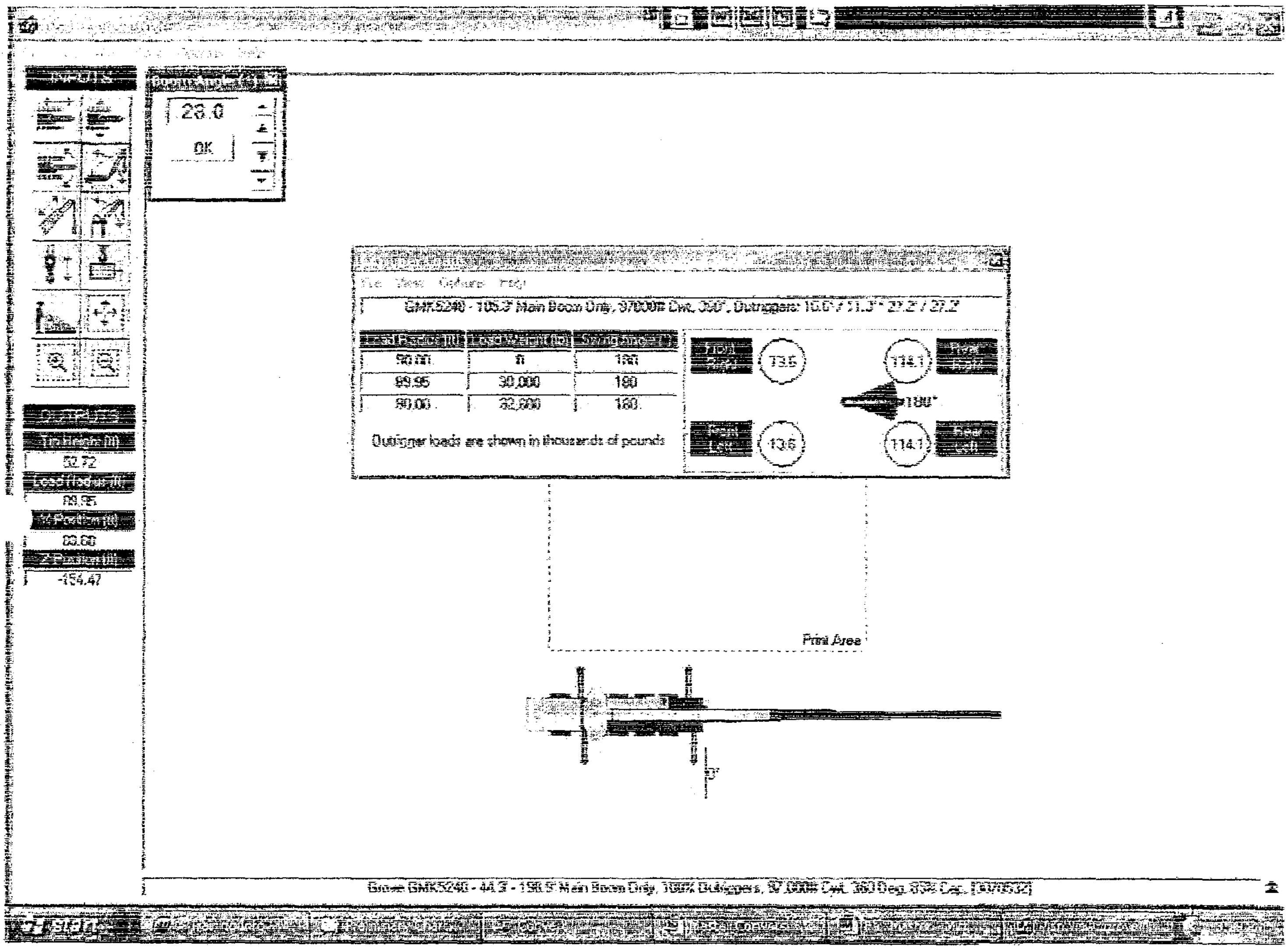

INPUTS
-45.0
OK
Carrier
OUTPUTS
52.72
-107.53
-147.27
GMK5240 - 105.3' Main Boom Only, 57000# Cwt, 360°, Outriggers: 16.6' / 11.3' * 27.2' / 27.2'
90.00
0
45
30,000
(45)
90.00
32,600
45
Outrigger loads are shown in thousands of pounds
55.8
5.0
125.0
65.5
45°
Print Area
Grove GMK5240 - 44.3' - 196.9' Main Boom Only, 100% Outriggers, 97,000# Cwt, 360 Deg, 85% Cap. [3070532]

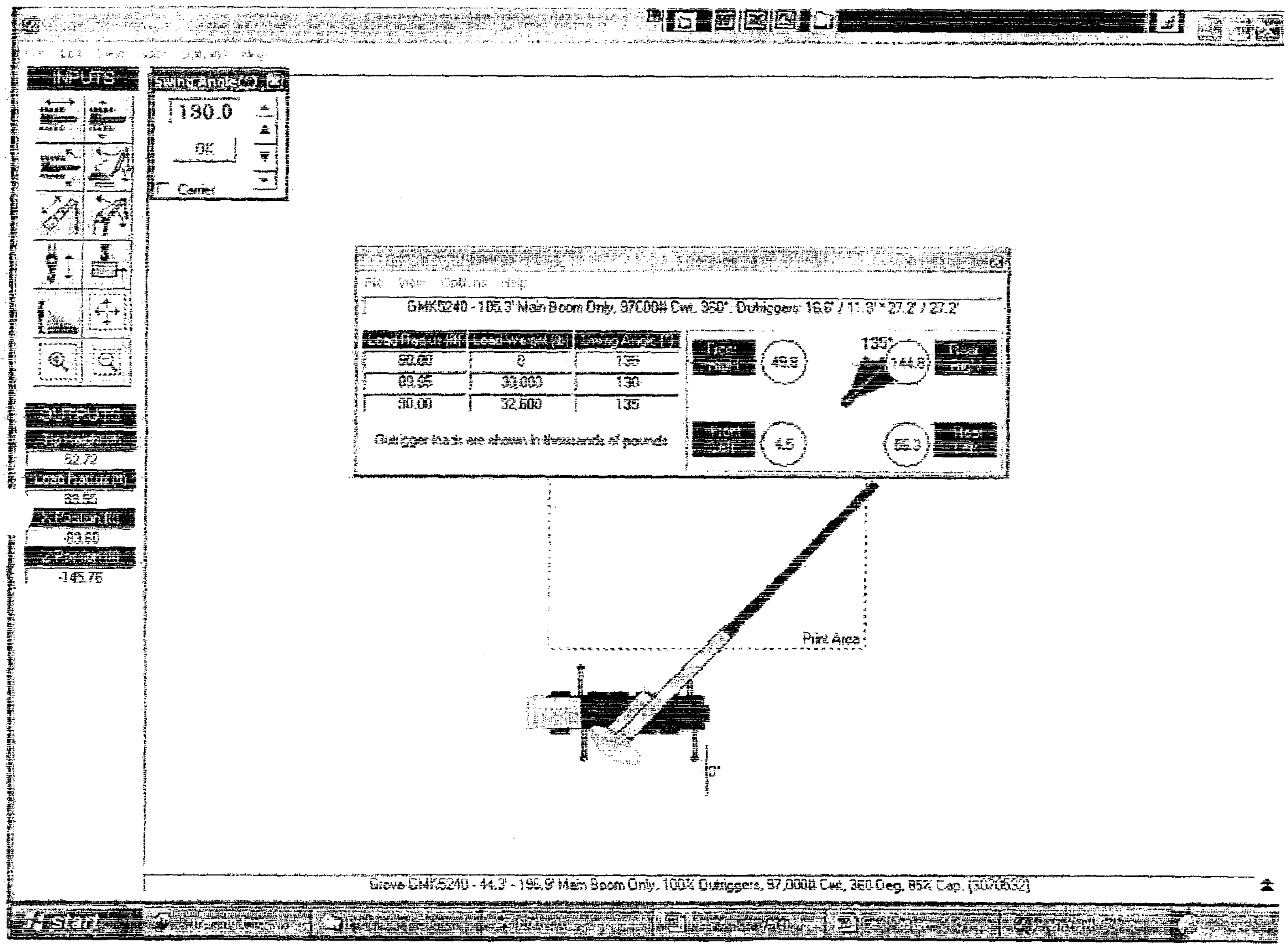

INPUTS
-90.0
OK
Carrier
52.72
89.95
91.90
Outrigger loads are shown in thousands of pounds
24.3
65.3
127.0
90°
Print Area
Grove GMK5240 - 44.3' - 196.9' Main Boom Only, 100% Outriggers, 97,000# Cwt, 360 Deg, 85% Cap. [3070532]
start


File Edit View Goto Options Help
INPUTS
-135.0
OK
Cancel
OUTPUTS
52.72
89.95
15.27
-53.61
Outrigger loads are shown in thousands of pounds
4.5
48.8
144.8
135°
Print Area
Grove GMK5240 - 44.3' - 196.9' Main Boom Only, 100% Outriggers, 97,000# Cwt, 360 Deg, 85% Cap, [3070532]

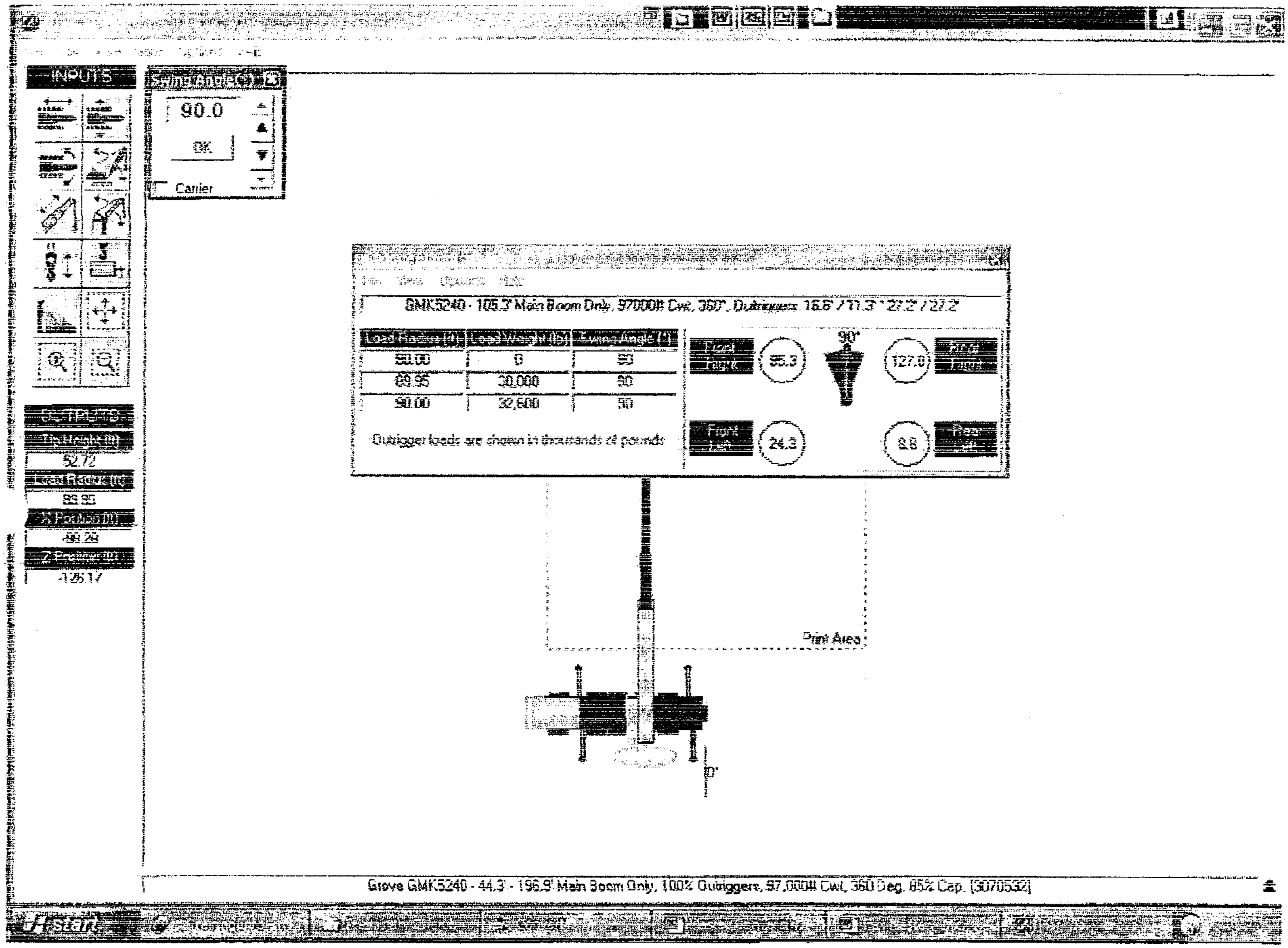
Grove Worldwide

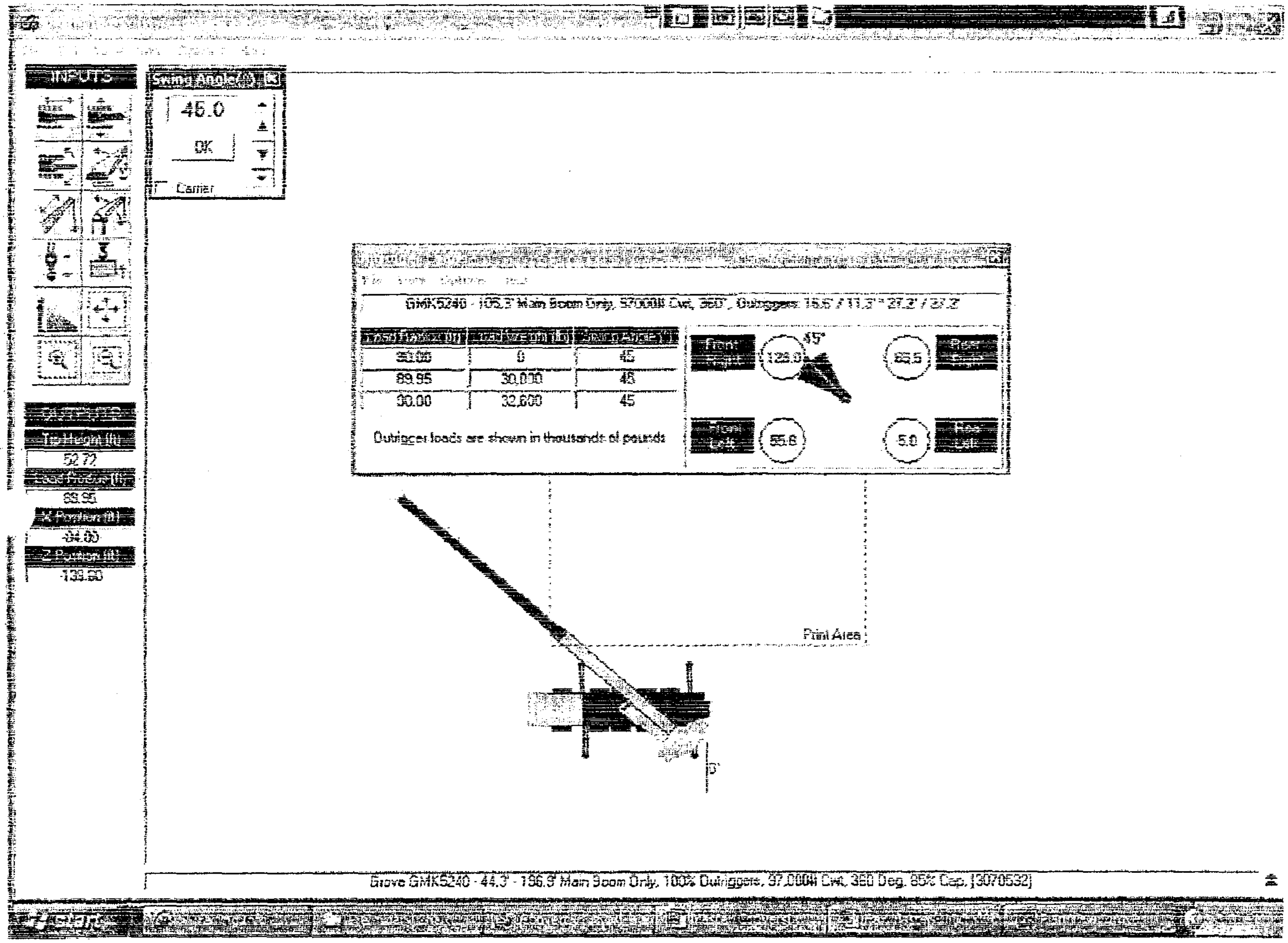
To Shawn	No. of Pages 16
Company	From Doug Myers
Dept.	Dept.
Fax No. 208 529 1614	Phone No.
	Fax No.

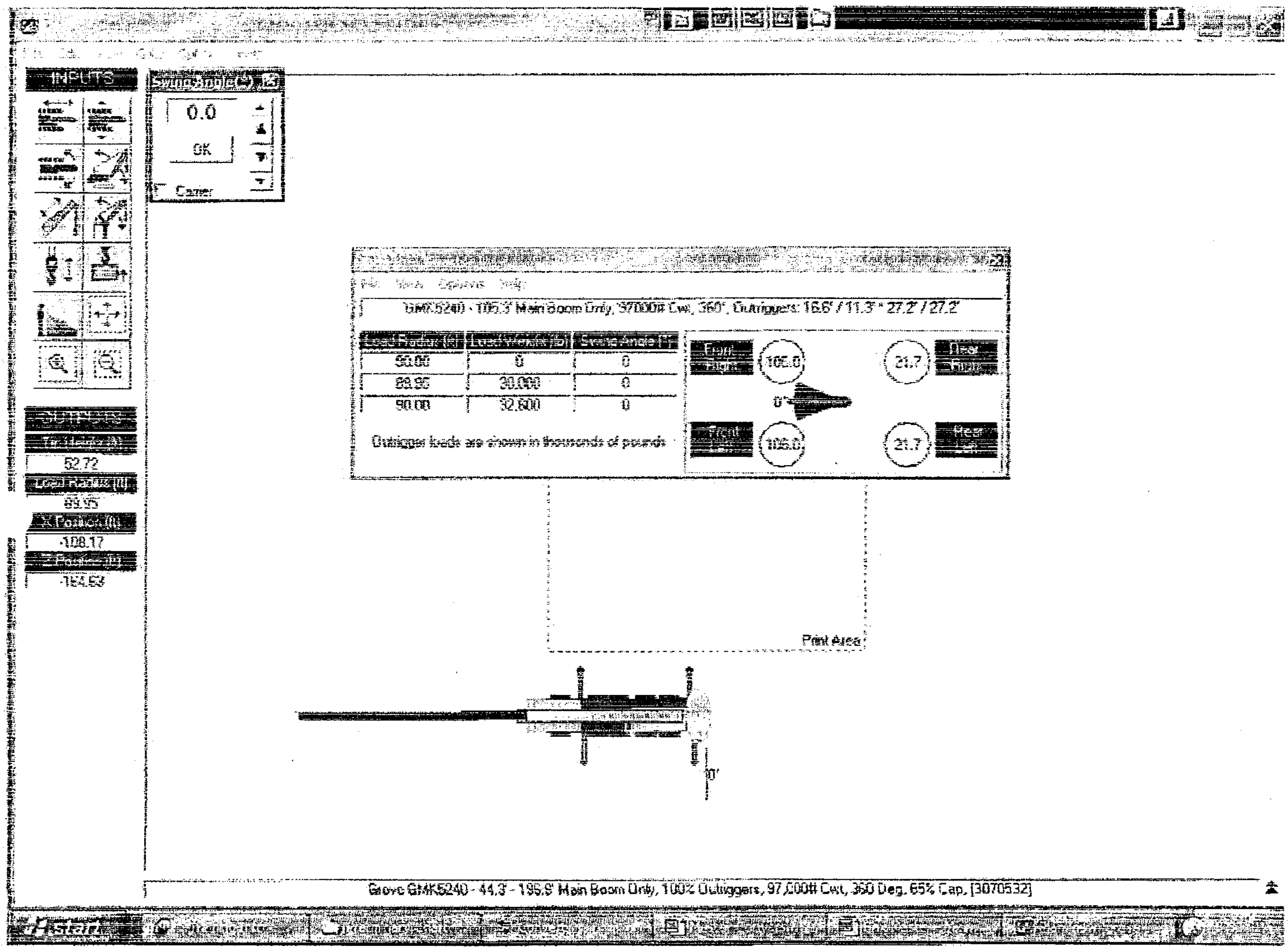




INPUTS
Swing Angle(°)
180.0
OK
Carrier
OUTPUTS
52.72
Load Radius (ft)
89.56
-89.60
-145.76
GMK5240 - 105.3' Main Boom Only, 97000# Cwt, 360°, Outriggers: 16.6' / 11.3' * 27.2' / 27.2'
90.00 0 135
89.56 30,000 130
90.00 32,600 135
Outrigger loads are shown in thousands of pounds
49.8
135°
144.8
4.5
55.3
Print Area
0°
Grove GMK5240 - 44.3' - 195.9' Main Boom Only, 100% Outriggers, 97,000# Cwt, 360 Deg, 85% Cap. [3070632]


INPUTS
Swing Angle(°)
90.0
OK
Carrier
GMK5240 - 105.3' Main Boom Only, 97000# Cwt, 360°, Outriggers 16.6' / 11.3' / 27.2' / 27.2'
Load Radius (ft)
Load Weight (lb)
Swing Angle (°)
90.00
0
90
89.95
30,000
90
90.00
32,600
90
Outrigger loads are shown in thousands of pounds
Front
Right
55.3
90°
127.0
Rear
Front
24.3
8.8
Rear
Left
Front
Left
Tip Height (ft)
52.72
Load Radius (ft)
89.95
X Position (ft)
Z Position (ft)
Print Area
0°
Grove GMK5240 - 44.3' - 196.9' Main Boom Only, 100% Outriggers, 97,000# Cwt, 360 Deg, 85% Cap. [3070532]
start


INPUTS
Swing Angle
45.0
OK
Carrier
Tip Height (ft)
52.72
89.95
GMK5240 - 105.3' Main Boom Only, 57000# Cwt, 360°, Outriggers 16.6' / 11.3' * 27.2' / 27.2'
0
45
89.95
30,000
45
30.00
32,600
45
Outrigger loads are shown in thousands of pounds
45°
128.0
65.5
55.6
5.0
Print Area
Grove GMK5240 - 44.3' - 196.9' Main Boom Only, 100% Outriggers, 37,000# Cwt, 360 Deg, 85% Cap. [3070532]


INPUTS
Swing Angle(°)
0.0
OK
Carrier
GMK5240 - 105.3' Main Boom Only, 97000# Cwt, 360°, Outriggers: 16.6' / 11.3' * 27.2' / 27.2'
Load Radius (ft)
Load Weight (lb)
Swing Angle (°)
50.00
0
0
88.95
30,000
0
90.00
32,600
0
Outrigger loads are shown in thousands of pounds
Front Right
105.0
Rear Right
21.7
0°
Front Left
106.0
Rear Left
21.7
OUTPUTS
Tip Height (ft)
52.72
Load Radius (ft)
88.95
X Position (ft)
-108.17
Z Position (ft)
-164.63
Print Area
0'
Grove GMK5240 - 44.3' - 196.9' Main Boom Only, 100% Outriggers, 97,000# Cwt, 360 Deg, 65% Cap, [3070532]

JOB PM 2A Crane Pad Design

SHEET NO. 1 OF 3

CALCULATED BY S Dustin DATE 6/16/03

CHECKED BY ______ DATE ______

SCALE ______

INTREPID Engineering Services, Inc.
A New Type of Engineering Company
501 West Broadway, Suite 200 Idaho Falls, ID 83402
(208) 529-5337

Purpose: Determine required outrigger pad area for crane

Given: Crane data as attached

Mfg. Pad loads for anticipated "critical" lifts as attached

Soil Data as attached

The soil bearing capacity as controlled by shear strength is 2500 psf (min value from spreadsheet).

According to the attached loading data from Grove Cranes, for a 30,000 load, the maximum point load that will be generated at any of the outriggers is 129,000 lbs.

Check soil bearing capacity for general and local shear failure Using a factor of safety of 3.0 and Terzaghi's method

$\gamma = 110$ lb/ft^3

$C = 2000$ lb/ft^2

$Q_{all} = 129$ kips

$\phi = 30°$

$N'_c = 20$

$N'_q = 9$

$N'_\gamma = 6$

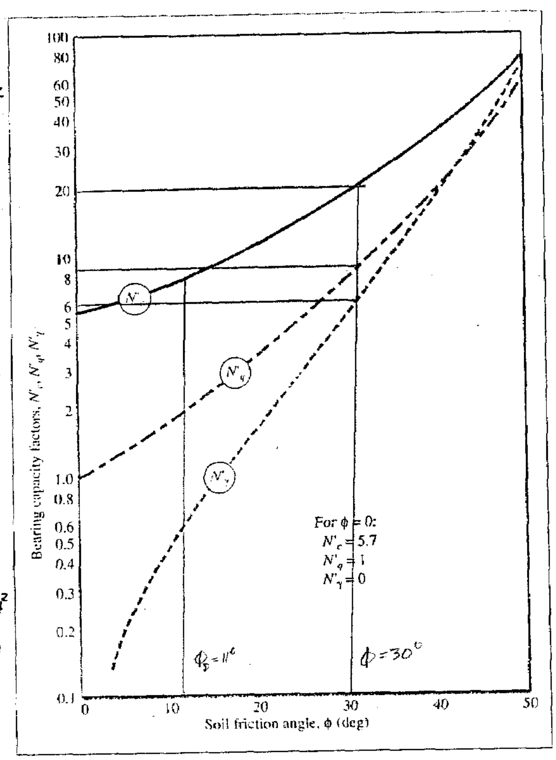
$N_c = 35$

$N_q = 22$

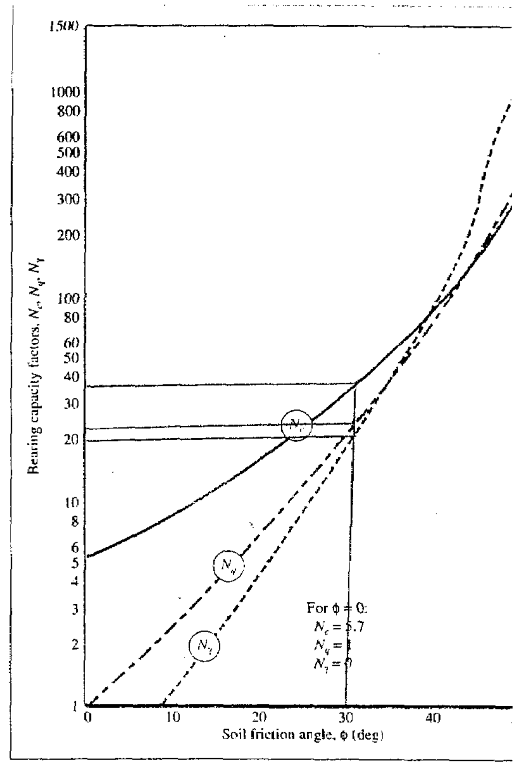
$N_\gamma = 20$

$C' = \frac{2}{3}C = 1333$ lb/ft^2

$q = 0$ (no overburden)



▼ FIGURE 11.8 Terzaghi's bearing capacity factors for local shear failure



▼ FIGURE 11.7 Terzaghi's bearing capacity factors for general shear failure

INTREPID Engineering Services, Inc.
A New Type of Engineering Company
501 West Broadway, Suite 200 Idaho Falls, ID 83402
(208) 529-5337

JOB PM 2A Crane Pad
SHEET NO. 2 OF 3
CALCULATED BY [illegible] DATE 6/16
CHECKED BY DATE
SCALE

Treat crane outrigger support pads as square footings.
Use equations 11.12 and 11.17 from Das for q_u and q'_u to solve for B_{min} for square pad:

GENERAL, q_u

$q_u = 1.3cN_c + qN_q^{\,0} + 0.4\gamma BN_\gamma$

$q_{all} = \frac{q_u}{3} = \frac{1}{3}(1.3cN_c + 0.4\gamma BN_\gamma)$

also $q_{all} = \frac{Q_{all}}{B^2} = \frac{129000}{B^2}$

$\therefore \frac{129000}{B^2} = \frac{1}{3}(1.3cN_c + 0.4\gamma BN_\gamma)$

$= \frac{1}{3}(1.3(2000)(35) + 0.4(110)(B)(20))$

$B^2\left(\frac{129000}{B^2} = 30303 + 293B\right)$

$129000 = 30303B^2 + 293B^3$

$293B^3 + 30303B^2 - 129000 = 0$

using www.1728.com/cubic.htm
where A = 293, B = 30303, C = 0, and
D = -129000

$x_1 = 2.0$ ft
$x_2 = -103$
$x_3 = -2.1$

LOCAL, q'_u

$q'_u = 1.3c'N'_c + qN_q^{\,0} + 0.4\gamma BN'_\gamma$

$q_{all} = \frac{1}{3}(1.3c'N'_c + 0.4\gamma BN'_\gamma)$

also $q_{all} = \frac{Q_{all}}{B^2} = \frac{129{,}000}{B^2}$

$\therefore \frac{129{,}000}{B^2} = \frac{1}{3}(1.3c'N'_c + 0.4\gamma BN'_\gamma)$

$= \frac{1}{3}(1.3(1333)(20) + 0.4(110)(B)(6))$

$B^2\left(\frac{129000}{B^2} = 11541 + 264(B)\right)$

$129000 = 11541B^2 + 264B^3$

$264B^3 + 11541B^2 - 129000 = 0$

using www.1728.com/cubic.htm
where A = 264, B = 11541, C = 0, D = -129000

$x_1 = 3.2$
$x_2 = -43.5$
$x_3 = -3.5$

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(208) 529-5337

JOB: PM 2A Crane Pad
SHEET NO. 3 OF 3
CALCULATED BY: S Dustin DATE: 6/16/03
CHECKED BY: DATE:
SCALE:

Check for gross allowable bearing capacity on shear strength of soil (Das, p 478-79) for FS = 3

C_d = developed cohesion $= \frac{c}{F_s}$

$= \frac{2000}{3} = 667 \text{ lbs/ft}^2$

$\tan \phi_d = \frac{\tan \phi}{F_s} = \frac{\tan 30}{3} = 0.1925$

$\phi_d = 11^\circ$

$q_{all} = 1.3(C_d)(N_c) + 0.4(\gamma)(B)(N_\gamma)$

$= 1.3(667)(7) + 0.4(110)(2.5)(0.7)$

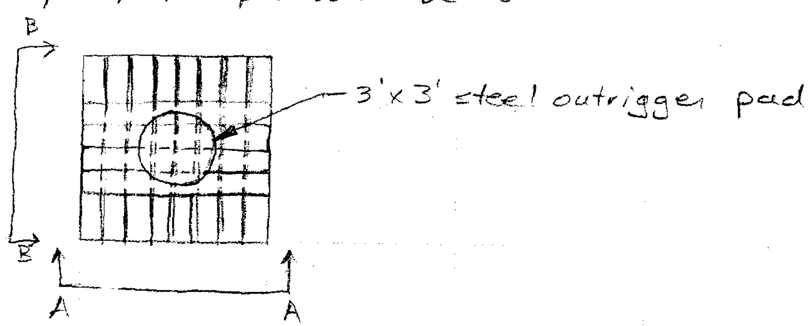
$q_{all} = 6147$

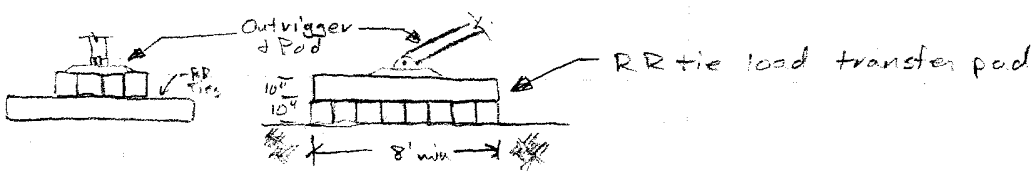
for 129 kip load,

$B^2 = \frac{129 \text{ kips}}{6.1 \text{ ksf}} = 21.1 \text{ ft}^2$

B = 4.6 ft ; This case controls

so for FS of 3, 4.6' x 4.6' square ftg is ok.
Given that we will be using rr ties (std. 8ft length) to construct pad, the pad will be 64 ft²:

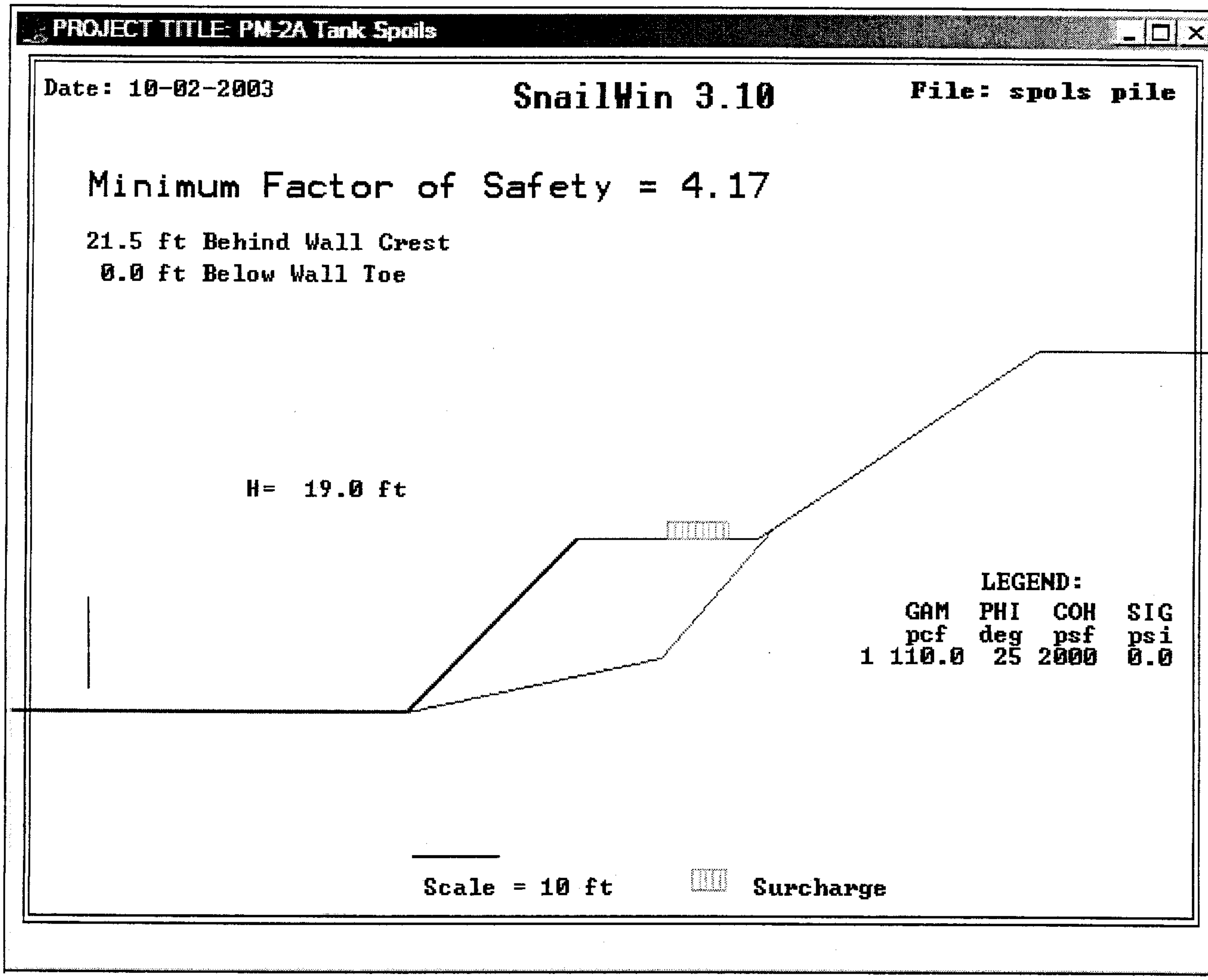




Sec B-B Sec A-A

Use 8' x 8' RR Tie load transfer pad as shown

Appendix E: SNAILZ Results


PROJECT TITLE: PM-2A Tank Spoils
Date: 10-02-2003
SnailWin 3.10
File: spols pile
Minimum Factor of Safety = 4.17
21.5 ft Behind Wall Crest
0.0 ft Below Wall Toe
H= 19.0 ft
LEGEND:
GAM PHI COH SIG
pcf deg psf psi
1 110.0 25 2000 0.0
Scale = 10 ft
Surcharge

```
***********************************************
*   CALIFORNIA DEPARTMENT OF TRANSPORTATION    *
*         ENGINEERING SERVICE CENTER           *
*    DIVISION OF MATERIALS AND FOUNDATIONS     *
*  Office of Roadway Geotechnical Engineering  *
*   Date: 10-02-2003          Time: 16:32:37   *
***********************************************
```

Project Identification - PM-2A Tank Spoils

--------- WALL GEOMETRY ---------

```
Vertical Wall Height            =  19.0 ft
Wall Batter                     =  45.0 degree
                                  Angle   Length
                                  (Deg)   (Feet)
First Slope from Wallcrest.   =   0.0     20.0
Second Slope from 1st slope.  =  33.7     37.0
Third Slope from 2nd slope.   =   0.0     50.0
Fourth Slope from 3rd slope.  =   0.0      0.0
Fifth Slope from 3rd slope.   =   0.0      0.0
Sixth Slope from 3rd slope.   =   0.0      0.0
Seventh Slope Angle.          =   0.0
```

--------- SLOPE BELOW THE WALL ---------

```
First Slope Angle below Toe.        =   0.0 degrees
First Slope Distance from Toe.      =  20.0 ft
Second Slope Angle.                 =   0.0 degrees
Second Slope Distance from Toe.     =   0.0 ft
Vertical Depth of Search.           =   7.5 ft
Number of Searches below wall Toe.  =   5
```

--------- SURCHARGE ---------

THE SURCHARGES IMPOSED ON THE SYSTEM ARE:

```
Begin Surcharge - Distance from toe =     29.0 ft
End Surcharge - Distance from toe   =     36.0 ft
Loading Intensity - Begin           =   3000.0 psf/ft
Loading Intensity - End             =   3000.0 psf/ft
```

--------- OPTION #1 ---------

Ultimate Punching shear, Bond & Yield Stress are used.

--------- SOIL PARAMETERS ---------

Soil Layer	Unit Weight (Pcf)	Friction Angle (Degree)	Cohesion Intercept (Psf)	Bond* Stress (Psi)	Coordinates of Boundary XS1 (ft)	YS1 (ft)	XS2 (ft)	YS2 (ft)
1	110.0	25.0	2000.0	0.0	0.0	0.0	0.0	0.0

* Ultimate bond Stress values also depend on BSF (Bond Stress Factor.)

--------- WATER SURFACE ---------

NO Water Table defined for this problem.

--------- SEARCH LIMIT ---------

The Search Limit is from 1.0 to 80.0 ft

You have chosen NOT TO LIMIT the search of failure planes to specific nodes.

--------- REINFORCEMENT PARAMETERS ---------

Number of Reinforcement Levels	=	0
Horizontal Spacing	=	5.0 ft
Diameter of Reinforcement Element	=	0.000 in
Yield Stress of Reinforcement	=	0.0 ksi
Diameter of Grouted Hole	=	0.0 in
Punching Shear	=	0.0 kips

--------- (For ALL Levels) ---------

Reinforcement Lengths	=	0.0 ft
Reinforcement Inclination	=	0.0 degrees
Vertical Spacing to First Level	=	0.0 ft
Vertical Spacing to Remaining Levels	=	0.0 ft

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
Toe	4.17	40.5	11.9	29.0	49.0	18.5

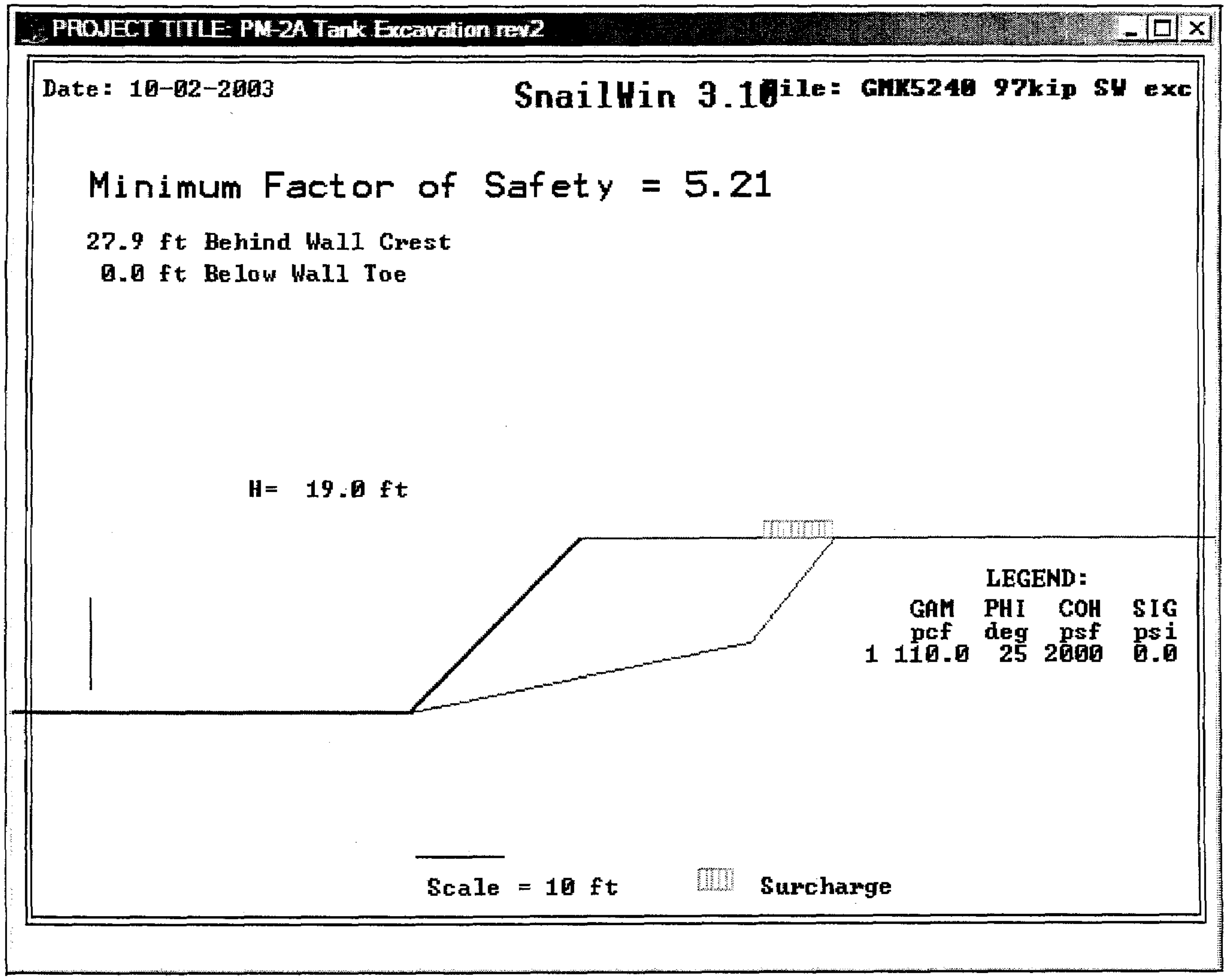
DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
1.50	4.50	40.5	12.8	29.1	51.1	19.3

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
3.00	4.79	40.5	13.7	29.2	53.0	20.2

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
4.50	5.02	40.5	0.0	16.2	45.2	34.5

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
6.00	5.19	40.5	0.0	12.2	42.5	38.5

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
7.50	5.38	40.5	0.0	12.2	44.1	39.5


PROJECT TITLE: PM-2A Tank Excavation rev2
Date: 10-02-2003
SnailWin 3.10
File: GMK5240 97kip SW exc
Minimum Factor of Safety = 5.21
27.9 ft Behind Wall Crest
0.0 ft Below Wall Toe
H= 19.0 ft
LEGEND:
GAM pcf
PHI deg
COH psf
SIG psi
1 110.0 25 2000 0.0
Scale = 10 ft
Surcharge

```
**************************************************
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*          ENGINEERING SERVICE CENTER            *
*     DIVISION OF MATERIALS AND FOUNDATIONS      *
*   Office of Roadway Geotechnical Engineering   *
*    Date: 10-02-2003          Time: 16:00:12    *
**************************************************
```

Project Identification - PM-2A Tank Excavation rev2

--------- WALL GEOMETRY ---------

```
Vertical Wall Height            =  19.0 ft
Wall Batter                     =  45.0 degree
                                   Angle   Length
                                   (Deg)   (Feet)
First Slope from Wallcrest.     =   0.0     50.0
Second Slope from 1st slope.    =   0.0      0.0
Third Slope from 2nd slope.     =   0.0      0.0
Fourth Slope from 3rd slope.    =   0.0      0.0
Fifth Slope from 3rd slope.     =   0.0      0.0
Sixth Slope from 3rd slope.     =   0.0      0.0
Seventh Slope Angle.            =   0.0
```

--------- SLOPE BELOW THE WALL ---------

```
First Slope Angle below Toe.         =   0.0 degrees
First Slope Distance from Toe.       =  20.0 ft
Second Slope Angle.                  =   0.0 degrees
Second Slope Distance from Toe.      =   0.0 ft
Vertical Depth of Search.            =   7.5 ft
Number of Searches below wall Toe.   =   5
```

--------- SURCHARGE ---------

THE SURCHARGES IMPOSED ON THE SYSTEM ARE:

```
Begin Surcharge - Distance from toe =     39.0 ft
End Surcharge - Distance from toe   =     47.0 ft
Loading Intensity - Begin           =   2015.0 psf/ft
Loading Intensity - End             =   2015.0 psf/ft
```

--------- OPTION #1 ---------

Ultimate Punching shear, Bond & Yield Stress are used.

--------- SOIL PARAMETERS ---------

Soil Layer	Unit Weight (Pcf)	Friction Angle (Degree)	Cohesion Intercept (Psf)	Bond* Stress (Psi)	Coordinates of Boundary XS1 (ft)	YS1 (ft)	XS2 (ft)	YS2 (ft)
1	110.0	25.0	2000.0	0.0	0.0	0.0	0.0	0.0

* Ultimate bond Stress values also depend on BSF (Bond Stress Factor.)

--------- WATER SURFACE ---------

NO Water Table defined for this problem.

--------- SEARCH LIMIT ---------

The Search Limit is from 0.0 to 50.0 ft

You have chosen NOT TO LIMIT the search of failure planes to specific nodes.

--------- REINFORCEMENT PARAMETERS ---------

Number of Reinforcement Levels	=	0
Horizontal Spacing	=	5.0 ft
Diameter of Reinforcement Element	=	0.000 in
Yield Stress of Reinforcement	=	0.0 ksi
Diameter of Grouted Hole	=	0.0 in
Punching Shear	=	0.0 kips

--------- (For ALL Levels) ---------

Reinforcement Lengths	=	0.0 ft
Reinforcement Inclination	=	0.0 degrees
Vertical Spacing to First Level	=	0.0 ft
Vertical Spacing to Remaining Levels	=	0.0 ft

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE		UPPER FAILURE PLANE	
			ANGLE (deg)	LENGTH (ft)	ANGLE (deg)	LENGTH (ft)
Toe	5.21	46.9	11.5	38.3	50.6	14.8

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE		UPPER FAILURE PLANE	
			ANGLE (deg)	LENGTH (ft)	ANGLE (deg)	LENGTH (ft)
1.50	5.48	46.9	12.3	38.4	52.7	15.5

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE		UPPER FAILURE PLANE	
			ANGLE (deg)	LENGTH (ft)	ANGLE (deg)	LENGTH (ft)
3.00	5.70	46.9	16.3	39.1	49.5	14.5

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE		UPPER FAILURE PLANE	
			ANGLE (deg)	LENGTH (ft)	ANGLE (deg)	LENGTH (ft)
4.50	5.86	46.9	0.0	14.1	35.6	40.4

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE		UPPER FAILURE PLANE	
			ANGLE (deg)	LENGTH (ft)	ANGLE (deg)	LENGTH (ft)
6.00	5.92	46.9	0.0	14.1	37.3	41.3

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE		UPPER FAILURE PLANE	
			ANGLE (deg)	LENGTH (ft)	ANGLE (deg)	LENGTH (ft)
7.50	5.96	46.9	0.0	14.1	38.9	42.2

PROJECT TITLE: PM-2A Tank Excavation rev2

Date: 10-02-2003

SnailWin 3.10

File: GMK5240 97kip SW ramp

Minimum Factor of Safety = 4.68

17.9 ft Behind Wall Crest

0.0 ft Below Wall Toe

H= 14.3 ft

LEGEND:

	GAM pcf	PHI deg	COH psf	SIG psi
1	110.0	25	2000	0.0

Scale = 10 ft

Surcharge

```
**************************************************
*    CALIFORNIA DEPARTMENT OF TRANSPORTATION     *
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*     DIVISION OF MATERIALS AND FOUNDATIONS      *
*   Office of Roadway Geotechnical Engineering   *
*    Date: 10-02-2003          Time: 15:58:39    *
**************************************************
```

Project Identification - PM-2A Tank Excavation rev2

--------- WALL GEOMETRY ---------

```
Vertical Wall Height          =  14.3 ft
Wall Batter                   =  45.0 degree
                                Angle   Length
                                (Deg)   (Feet)
First Slope from Wallcrest.   =  0.0     50.0
Second Slope from 1st slope.  =  0.0      0.0
Third Slope from 2nd slope.   =  0.0      0.0
Fourth Slope from 3rd slope.  =  0.0      0.0
Fifth Slope from 3rd slope.   =  0.0      0.0
Sixth Slope from 3rd slope.   =  0.0      0.0
Seventh Slope Angle.          =  0.0
```

--------- SLOPE BELOW THE WALL ---------

```
First Slope Angle below Toe.        =   0.0 degrees
First Slope Distance from Toe.      =  20.0 ft
Second Slope Angle.                 =   0.0 degrees
Second Slope Distance from Toe.     =   0.0 ft
Vertical Depth of Search.           =   7.5 ft
Number of Searches below wall Toe.  =   5
```

--------- SURCHARGE ---------

THE SURCHARGES IMPOSED ON THE SYSTEM ARE:

```
Begin Surcharge - Distance from toe =      23.7 ft
End Surcharge - Distance from toe   =      31.7 ft
Loading Intensity - Begin           =    2015.0 psf/ft
Loading Intensity - End             =    2015.0 psf/ft
```

--------- OPTION #1 ---------

Ultimate Punching shear, Bond & Yield Stress are used.

--------- SOIL PARAMETERS ---------

Soil Layer	Unit Weight (Pcf)	Friction Angle (Degree)	Cohesion Intercept (Psf)	Bond* Stress (Psi)	Coordinates of Boundary XS1 (ft)	YS1 (ft)	XS2 (ft)	YS2 (ft)
1	110.0	25.0	2000.0	0.0	0.0	0.0	0.0	0.0

* Ultimate bond Stress values also depend on BSF (Bond Stress Factor.)

--------- WATER SURFACE ---------

NO Water Table defined for this problem.

--------- SEARCH LIMIT ---------

The Search Limit is from 0.0 to 50.0 ft

You have chosen NOT TO LIMIT the search of failure planes to specific nodes.

--------- REINFORCEMENT PARAMETERS ---------

Number of Reinforcement Levels	= 0
Horizontal Spacing	= 5.0 ft
Diameter of Reinforcement Element	= 0.000 in
Yield Stress of Reinforcement	= 0.0 ksi
Diameter of Grouted Hole	= 0.0 in
Punching Shear	= 0.0 kips

--------- (For ALL Levels) ---------

Reinforcement Lengths	= 0.0 ft
Reinforcement Inclination	= 0.0 degrees
Vertical Spacing to First Level	= 0.0 ft
Vertical Spacing to Remaining Levels	= 0.0 ft

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE		UPPER FAILURE PLANE	
			ANGLE (deg)	LENGTH (ft)	ANGLE (deg)	LENGTH (ft)
Toe	4.68	32.2	7.2	22.7	49.9	15.0

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE		UPPER FAILURE PLANE	
			ANGLE (deg)	LENGTH (ft)	ANGLE (deg)	LENGTH (ft)
1.50	5.18	32.2	11.9	23.0	48.9	14.7

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE		UPPER FAILURE PLANE	
			ANGLE (deg)	LENGTH (ft)	ANGLE (deg)	LENGTH (ft)
3.00	5.60	32.2	13.0	23.1	51.5	15.5

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE		UPPER FAILURE PLANE	
			ANGLE (deg)	LENGTH (ft)	ANGLE (deg)	LENGTH (ft)
4.50	5.95	32.2	0.0	12.9	44.3	26.9

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE		UPPER FAILURE PLANE	
			ANGLE (deg)	LENGTH (ft)	ANGLE (deg)	LENGTH (ft)
6.00	6.24	32.2	0.0	9.6	42.1	30.3

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE		UPPER FAILURE PLANE	
			ANGLE (deg)	LENGTH (ft)	ANGLE (deg)	LENGTH (ft)
7.50	6.49	32.2	0.0	9.6	44.1	31.3

Appendix E: CFR Title 29, Chapter XVII, Section 1926.652

[Code of Federal Regulations]
[Title 29, Volume 8]
[Revised as of July 1, 2002]
From the U.S. Government Printing Office via GPO Access
[CITE: 29CFR1926.652]

[Page 377-410]

TITLE 29--LABOR

CHAPTER XVII--OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION, DEPARTMENT OF LABOR

PART 1926--SAFETY AND HEALTH REGULATIONS FOR CONSTRUCTION--Table of Contents

Subpart P--Excavations

Sec. 1926.**652** Requirements for protective systems.

(a) Protection of employees in excavations. (1) Each employee in an excavation shall be protected from cave-ins by an adequate protective system designed in accordance with paragraph (b) or (c) of this section except when:

(i) Excavations are made entirely in stable rock; or

(ii) Excavations are less than 5 feet (1.52m) in depth and examination of the ground by a competent person provides no indication of a potential cave-in.

(2) Protective systems shall have the capacity to resist without failure all loads that are intended or could reasonably be expected to be applied or transmitted to the system.

(b) Design of sloping and benching systems. The slopes and configurations of sloping and benching systems shall be selected and constructed by the employer or his designee and shall be in accordance with the requirements of paragraph (b)(1); or, in the alternative, paragraph (b)(2); or, in the alternative, paragraph (b)(3), or, in the alternative, paragraph (b)(4), as follows:

(1) Option (1)--Allowable configurations and slopes. (i) Excavations shall be sloped at an angle not steeper than one and one-half horizontal to one vertical

[[Page 378]]

(34 degrees measured from the horizontal), unless the employer uses one of the other options listed below.

(ii) Slopes specified in paragraph (b)(1)(i) of this section, shall be excavated to form configurations that are in accordance with the slopes shown for Type C soil in Appendix B to this subpart.

(2) Option (2)--Determination of slopes and configurations using Appendices A and B. Maximum allowable slopes, and allowable configurations for sloping and benching systems, shall be determined in accordance with the conditions and requirements set forth in appendices A and B to this subpart.

(3) Option (3)--Designs using other tabulated data. (i) Designs of sloping or benching systems shall be selected from and be in accordance with tabulated data, such as tables and charts.

(ii) The tabulated data shall be in written form and shall include all of the following:

(A) Identification of the parameters that affect the selection of a sloping or benching system drawn from such data;

(B) Identification of the limits of use of the data, to include the

magnitude and configuration of slopes determined to be safe;
(C) Explanatory information as may be necessary to aid the user in making a correct selection of a protective system from the data.
(iii) At least one copy of the tabulated data which identifies the registered professional engineer who approved the data, shall be maintained at the jobsite during construction of the protective system. After that time the data may be stored off the jobsite, but a copy of the data shall be made available to the Secretary upon request.
(4) Option (4)--Design by a registered professional engineer. (i) Sloping and benching systems not utilizing Option (1) or Option (2) or Option (3) under paragraph (b) of this section shall be approved by a registered professional engineer.
(ii) Designs shall be in written form and shall include at least the following:
(A) The magnitude of the slopes that were determined to be safe for the particular project;
(B) The configurations that were determined to be safe for the particular project; and
(C) The identity of the registered professional engineer approving the design.
(iii) At least one copy of the design shall be maintained at the jobsite while the slope is being constructed. After that time the design need not be at the jobsite, but a copy shall be made available to the Secretary upon request.
(c) Design of support systems, shield systems, and other protective systems. Designs of support systems shield systems, and other protective systems shall be selected and constructed by the employer or his designee and shall be in accordance with the requirements of paragraph (c)(1); or, in the alternative, paragraph (c)(2); or, in the alternative, paragraph (c)(3); or, in the alternative, paragraph (c)(4) as follows:
(1) Option (1)--Designs using appendices A, C and D. Designs for timber shoring in trenches shall be determined in accordance with the conditions and requirements set forth in appendices A and C to this subpart. Designs for aluminum hydraulic shoring shall be in accordance with paragraph (c)(2) of this section, but if manufacturer's tabulated data cannot be utilized, designs shall be in accordance with appendix D.
(2) Option (2)--Designs Using Manufacturer's Tabulated Data. (i) Design of support systems, shield systems, or other protective systems that are drawn from manufacturer's tabulated data shall be in accordance with all specifications, recommendations, and limitations issued or made by the manufacturer.
(ii) Deviation from the specifications, recommendations, and limitations issued or made by the manufacturer shall only be allowed after the manufacturer issues specific written approval.
(iii) Manufacturer's specifications, recommendations, and limitations, and manufacturer's approval to deviate from the specifications, recommendations, and limitations shall be in written form at the jobsite during construction of the protective system. After that time this data may be stored off the jobsite, but a copy shall

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be made available to the Secretary upon request.
(3) Option (3)--Designs using other tabulated data. (i) Designs of support systems, shield systems, or other protective systems shall be selected from and be in accordance with tabulated data, such as tables and charts.
(ii) The tabulated data shall be in written form and include all of the following:

(A) Identification of the parameters that affect the selection of a protective system drawn from such data;

(B) Identification of the limits of use of the data;

(C) Explanatory information as may be necessary to aid the user in making a correct selection of a protective system from the data.

(iii) At least one copy of the tabulated data, which identifies the registered professional engineer who approved the data, shall be maintained at the jobsite during construction of the protective system. After that time the data may be stored off the jobsite, but a copy of the data shall be made available to the Secretary upon request.

(4) Option (4)--Design by a registered professional engineer. (i) Support systems, shield systems, and other protective systems not utilizing Option 1, Option 2 or Option 3, above, shall be approved by a registered professional engineer.

(ii) Designs shall be in written form and shall include the following:

(A) A plan indicating the sizes, types, and configurations of the materials to be used in the protective system; and

(B) The identity of the registered professional engineer approving the design.

(iii) At least one copy of the design shall be maintained at the jobsite during construction of the protective system. After that time, the design may be stored off the jobsite, but a copy of the design shall be made available to the Secretary upon request.

(d) Materials and equipment. (1) Materials and equipment used for protective systems shall be free from damage or defects that might impair their proper function.

(2) Manufactured materials and equipment used for protective systems shall be used and maintained in a manner that is consistent with the recommendations of the manufacturer, and in a manner that will prevent employee exposure to hazards.

(3) When material or equipment that is used for protective systems is damaged, a competent person shall examine the material or equipment and evaluate its suitability for continued use. If the competent person cannot assure the material or equipment is able to support the intended loads or is otherwise suitable for safe use, then such material or equipment shall be removed from service, and shall be evaluated and approved by a registered professional engineer before being returned to service.

(e) Installation and removal of support--(1) General. (i) Members of support systems shall be securely connected together to prevent sliding, falling, kickouts, or other predictable failure.

(ii) Support systems shall be installed and removed in a manner that protects employees from cave-ins, structural collapses, or from being struck by members of the support system.

(iii) Individual members of support systems shall not be subjected to loads exceeding those which those members were designed to withstand.

(iv) Before temporary removal of individual members begins, additional precautions shall be taken to ensure the safety of employees, such as installing other structural members to carry the loads imposed on the support system.

(v) Removal shall begin at, and progress from, the bottom of the excavation. Members shall be released slowly so as to note any indication of possible failure of the remaining members of the structure or possible cave-in of the sides of the excavation.

(vi) Backfilling shall progress together with the removal of support systems from excavations.

(2) Additional requirements for support systems for trench excavations. (i) Excavation of material to a level no greater than 2 feet (.61 m) below the bottom of the members of a support system shall

be permitted, but only if the system is designed to resist the forces calculated for the full depth of the trench, and

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there are no indications while the trench is open of a possible loss of soil from behind or below the bottom of the support system.

(ii) Installation of a support system shall be closely coordinated with the excavation of trenches.

(f) Sloping and benching systems. Employees shall not be permitted to work on the faces of sloped or benched excavations at levels above other employees except when employees at the lower levels are adequately protected from the hazard of falling, rolling, or sliding material or equipment.

(g) Shield systems--(1) General. (i) Shield systems shall not be subjected to loads exceeding those which the system was designed to withstand.

(ii) Shields shall be installed in a manner to restrict lateral or other hazardous movement of the shield in the event of the application of sudden lateral loads.

(iii) Employees shall be protected from the hazard of cave-ins when entering or exiting the areas protected by shields.

(iv) Employees shall not be allowed in shields when shields are being installed, removed, or moved vertically.

(2) Additional requirement for shield systems used in trench excavations. Excavations of earth material to a level not greater than 2 feet (.61 m) below the bottom of a shield shall be permitted, but only if the shield is designed to resist the forces calculated for the full depth of the trench, and there are no indications while the trench is open of a possible loss of soil from behind or below the bottom of the shield.

Appendix A to Subpart P of Part 1926--Soil Classification

(a) Scope and application--(1) Scope. This appendix describes a method of classifying soil and rock deposits based on site and environmental conditions, and on the structure and composition of the earth deposits. The appendix contains definitions, sets forth requirements, and describes acceptable visual and manual tests for use in classifying soils.

(2) Application. This appendix applies when a sloping or benching system is designed in accordance with the requirements set forth in Sec. 1926.**652**(b)(2) as a method of protection for employees from cave-ins. This appendix also applies when timber shoring for excavations is designed as a method of protection from cave-ins in accordance with appendix C to subpart P of part 1926, and when aluminum hydraulic shoring is designed in accordance with appendix D. This Appendix also applies if other protective systems are designed and selected for use from data prepared in accordance with the requirements set forth in Sec. 1926.**652**(c), and the use of the data is predicated on the use of the soil classification system set forth in this appendix.

(b) Definitions. The definitions and examples given below are based on, in whole or in part, the following: American Society for Testing Materials (ASTM) Standards D653-85 and D2488; The Unified Soils Classification System, The U.S. Department of Agriculture (USDA) Textural Classification Scheme; and The National Bureau of Standards Report BSS-121.

Cemented soil means a soil in which the particles are held together by a chemical agent, such as calcium carbonate, such that a hand-size sample cannot be crushed into powder or individual soil particles by finger pressure.

Cohesive soil means clay (fine grained soil), or soil with a high clay content, which has cohesive strength. Cohesive soil does not crumble, can be excavated with vertical sideslopes, and is plastic when moist. Cohesive soil is hard to break up when dry, and exhibits significant cohesion when submerged. Cohesive soils include clayey silt, sandy clay, silty clay, clay and organic clay.

Dry soil means soil that does not exhibit visible signs of moisture content.

Fissured means a soil material that has a tendency to break along definite planes of fracture with little resistance, or a material that exhibits open cracks, such as tension cracks, in an exposed surface.

Granular soil means gravel, sand, or silt, (coarse grained soil) with little or no clay content. Granular soil has no cohesive strength. Some moist granular soils exhibit apparent cohesion. Granular soil cannot be molded when moist and crumbles easily when dry.

Layered system means two or more distinctly different soil or rock types arranged in layers. Micaceous seams or weakened planes in rock or shale are considered layered.

Moist soil means a condition in which a soil looks and feels damp. Moist cohesive soil can easily be shaped into a ball and rolled into small diameter threads before crumbling. Moist granular soil that contains some cohesive material will exhibit signs of cohesion between particles.

Plastic means a property of a soil which allows the soil to be deformed or molded without cracking, or appreciable volume change.

Saturated soil means a soil in which the voids are filled with water. Saturation does not require flow. Saturation, or near saturation, is necessary for the proper use of instruments such as a pocket penetrometer or sheer vane.

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Soil classification system means, for the purpose of this subpart, a method of categorizing soil and rock deposits in a hierarchy of Stable Rock, Type A, Type B, and Type C, in decreasing order of stability. The categories are determined based on an analysis of the properties and performance characteristics of the deposits and the environmental conditions of exposure.

Stable rock means natural solid mineral matter that can be excavated with vertical sides and remain intact while exposed.

Submerged soil means soil which is underwater or is free seeping.

Type A means cohesive soils with an unconfined compressive strength of 1.5 ton per square foot (tsf) (144 kPa) or greater. Examples of cohesive soils are: clay, silty clay, sandy clay, clay loam and, in some cases, silty clay loam and sandy clay loam. Cemented soils such as caliche and hardpan are also considered Type A. However, no soil is Type A if:

(i) The soil is fissured; or

(ii) The soil is subject to vibration from heavy traffic, pile driving, or similar effects; or

(iii) The soil has been previously disturbed; or

(iv) The soil is part of a sloped, layered system where the layers dip into the excavation on a slope of four horizontal to one vertical (4H:1V) or greater; or

(v) The material is subject to other factors that would require it to be classified as a less stable material.

Type B means:

(i) Cohesive soil with an unconfined compressive strength greater than 0.5 tsf (48 kPa) but less than 1.5 tsf (144 kPa); or

(ii) Granular cohesionless soils including: angular gravel (similar to crushed rock), silt, silt loam, sandy loam and, in some cases, silty clay loam and sandy clay loam.

(iii) Previously disturbed soils except those which would otherwise be classed as Type C soil.

(iv) Soil that meets the unconfined compressive strength or cementation requirements for Type A, but is fissured or subject to vibration; or

(v) Dry rock that is not stable; or

(vi) Material that is part of a sloped, layered system where the layers dip into the excavation on a slope less steep than four horizontal to one vertical (4H:1V), but only if the material would otherwise be classified as Type B.

Type C means:

(i) Cohesive soil with an unconfined compressive strength of 0.5 tsf (48 kPa) or less; or

(ii) Granular soils including gravel, sand, and loamy sand; or

(iii) Submerged soil or soil from which water is freely seeping; or

(iv) Submerged rock that is not stable, or

(v) Material in a sloped, layered system where the layers dip into the excavation or a slope of four horizontal to one vertical (4H:1V) or steeper.

Unconfined compressive strength means the load per unit area at which a soil will fail in compression. It can be determined by laboratory testing, or estimated in the field using a pocket penetrometer, by thumb penetration tests, and other methods.

Wet soil means soil that contains significantly more moisture than moist soil, but in such a range of values that cohesive material will slump or begin to flow when vibrated. Granular material that would exhibit cohesive properties when moist will lose those cohesive properties when wet.

(c) Requirements--(1) Classification of soil and rock deposits. Each soil and rock deposit shall be classified by a competent person as Stable Rock, Type A, Type B, or Type C in accordance with the definitions set forth in paragraph (b) of this appendix.

(2) Basis of classification. The classification of the deposits shall be made based on the results of at least one visual and at least one manual analysis. Such analyses shall be conducted by a competent person using tests described in paragraph (d) below, or in other recognized methods of soil classification and testing such as those adopted by the America Society for Testing Materials, or the U.S. Department of Agriculture textural classification system.

(3) Visual and manual analyses. The visual and manual analyses, such as those noted as being acceptable in paragraph (d) of this appendix, shall be designed and conducted to provide sufficient quantitative and qualitative information as may be necessary to identify properly the properties, factors, and conditions affecting the classification of the deposits.

(4) Layered systems. In a layered system, the system shall be classified in accordance with its weakest layer. However, each layer may be classified individually where a more stable layer lies under a less stable layer.

(5) Reclassification. If, after classifying a deposit, the properties, factors, or conditions affecting its classification change in any way, the changes shall be evaluated by a competent person. The deposit shall be reclassified as necessary to reflect the changed circumstances.

(d) Acceptable visual and manual tests.--(1) Visual tests. Visual analysis is conducted to determine qualitative information regarding the excavation site in general, the soil adjacent to the excavation, the soil forming the sides of the open excavation, and the soil taken as samples from excavated material.

(i) Observe samples of soil that are excavated and soil in the sides of the excavation. Estimate the range of particle sizes and the relative amounts of the particle sizes. Soil that is primarily composed of fine-grained

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material is cohesive material. Soil composed primarily of coarse-grained sand or gravel is granular material.

(ii) Observe soil as it is excavated. Soil that remains in clumps when excavated is cohesive. Soil that breaks up easily and does not stay in clumps is granular.

(iii) Observe the side of the opened excavation and the surface area adjacent to the excavation. Crack-like openings such as tension cracks could indicate fissured material. If chunks of soil spall off a vertical side, the soil could be fissured. Small spalls are evidence of moving ground and are indications of potentially hazardous situations.

(iv) Observe the area adjacent to the excavation and the excavation itself for evidence of existing utility and other underground structures, and to identify previously disturbed soil.

(v) Observe the opened side of the excavation to identify layered systems. Examine layered systems to identify if the layers slope toward the excavation. Estimate the degree of slope of the layers.

(vi) Observe the area adjacent to the excavation and the sides of the opened excavation for evidence of surface water, water seeping from the sides of the excavation, or the location of the level of the water table.

(vii) Observe the area adjacent to the excavation and the area within the excavation for sources of vibration that may affect the stability of the excavation face.

(2) Manual tests. Manual analysis of soil samples is conducted to determine quantitative as well as qualitative properties of soil and to provide more information in order to classify soil properly.

(i) Plasticity. Mold a moist or wet sample of soil into a ball and attempt to roll it into threads as thin as \1/8\-inch in diameter. Cohesive material can be successfully rolled into threads without crumbling. For example, if at least a two inch (50 mm) length of \1/8\-inch thread can be held on one end without tearing, the soil is cohesive.

(ii) Dry strength. If the soil is dry and crumbles on its own or with moderate pressure into individual grains or fine powder, it is granular (any combination of gravel, sand, or silt). If the soil is dry and falls into clumps which break up into smaller clumps, but the smaller clumps can only be broken up with difficulty, it may be clay in any combination with gravel, sand or silt. If the dry soil breaks into clumps which do not break up into small clumps and which can only be broken with difficulty, and there is no visual indication the soil is fissured, the soil may be considered unfissured.

(iii) Thumb penetration. The thumb penetration test can be used to estimate the unconfined compressive strength of cohesive soils. (This test is based on the thumb penetration test described in American Society for Testing and Materials (ASTM) Standard designation D2488--``Standard Recommended Practice for Description of Soils (Visual--Manual Procedure).'') Type A soils with an unconfined compressive strength of 1.5 tsf can be readily indented by the thumb; however, they can be penetrated by the thumb only with very great effort. Type C soils with an unconfined compressive strength of 0.5 tsf can be easily penetrated several inches by the thumb, and can be molded by light finger pressure.

This test should be conducted on an undisturbed soil sample, such as a large clump of spoil, as soon as practicable after excavation to keep to a miminum the effects of exposure to drying influences. If the excavation is later exposed to wetting influences (rain, flooding), the classification of the soil must be changed accordingly.

(iv) Other strength tests. Estimates of unconfined compressive strength of soils can also be obtained by use of a pocket penetrometer or by using a hand-operated shearvane.

(v) Drying test. The basic purpose of the drying test is to differentiate between cohesive material with fissures, unfissured cohesive material, and granular material. The procedure for the drying test involves drying a sample of soil that is approximately one inch thick (2.54 cm) and six inches (15.24 cm) in diameter until it is thoroughly dry:

(A) If the sample develops cracks as it dries, significant fissures are indicated.

(B) Samples that dry without cracking are to be broken by hand. If considerable force is necessary to break a sample, the soil has significant cohesive material content. The soil can be classified as a unfissured cohesive material and the unconfined compressive strength should be determined.

(C) If a sample breaks easily by hand, it is either a fissured cohesive material or a granular material. To distinguish between the two, pulverize the dried clumps of the sample by hand or by stepping on them. If the clumps do not pulverize easily, the material is cohesive with fissures. If they pulverize easily into very small fragments, the material is granular.

Appendix B to Subpart P of Part 1926--Sloping and Benching

(a) Scope and application. This appendix contains specifications for sloping and benching when used as methods of protecting employees working in excavations from cave-ins. The requirements of this appendix apply when the design of sloping and benching protective systems is to be performed in accordance with the requirements set forth in Sec. 1926.**652**(b)(2).

(b) Definitions.

Actual slope means the slope to which an excavation face is excavated.

Distress means that the soil is in a condition where a cave-in is imminent or is likely

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to occur. Distress is evidenced by such phenomena as the development of fissures in the face of or adjacent to an open excavation; the subsidence of the edge of an excavation; the slumping of material from the face or the bulging or heaving of material from the bottom of an excavation; the spalling of material from the face of an excavation; and ravelling, i.e., small amounts of material such as pebbles or little clumps of material suddenly separating from the face of an excavation and trickling or rolling down into the excavation.

Maximum allowable slope means the steepest incline of an excavation face that is acceptable for the most favorable site conditions as protection against cave-ins, and is expressed as the ratio of horizontal distance to vertical rise (H:V).

Short term exposure means a period of time less than or equal to 24 hours that an excavation is open.

(c) Requirements--(1) Soil classification. Soil and rock deposits shall be classified in accordance with appendix A to subpart P of part

1926.

(2) Maximum allowable slope. The maximum allowable slope for a soil or rock deposit shall be determined from Table B-1 of this appendix.

(3) Actual slope. (i) The actual slope shall not be steeper than the maximum allowable slope.

(ii) The actual slope shall be less steep than the maximum allowable slope, when there are signs of distress. If that situation occurs, the slope shall be cut back to an actual slope which is at least \1/2\ horizontal to one vertical (\1/2\H:1V) less steep than the maximum allowable slope.

(iii) When surcharge loads from stored material or equipment, operating equipment, or traffic are present, a competent person shall determine the degree to which the actual slope must be reduced below the maximum allowable slope, and shall assure that such reduction is achieved. Surcharge loads from adjacent structures shall be evaluated in accordance with Sec. 1926.651(i).

(4) Configurations. Configurations of sloping and benching systems shall be in accordance with Figure B-1.

[GRAPHIC] [TIFF OMITTED] TC30OC91.016

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Figure B-1

Slope Configurations

(All slopes stated below are in the horizontal to vertical ratio)

B-1.1 Excavations made in Type A soil.

1. All simple slope excavation 20 feet or less in depth shall have a maximum allowable slope of \3/4\:1.

[GRAPHIC] [TIFF OMITTED] TC30OC91.017

Simple Slope--General

Exception: Simple slope excavations which are open 24 hours or less (short term) and which are 12 feet or less in depth shall have a maximum allowable slope of \1/2\:1.

[GRAPHIC] [TIFF OMITTED] TC30OC91.018

Simple Slope--Short Term

2. All benched excavations 20 feet or less in depth shall have a maximum allowable slope of \3/4\ to 1 and maximum bench dimensions as follows:

[GRAPHIC] [TIFF OMITTED] TC30OC91.019

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Simple Bench

[GRAPHIC] [TIFF OMITTED] TC30OC91.020

Multiple Bench

3. All excavations 8 feet or less in depth which have unsupported vertically sided lower portions shall have a maximum vertical side of 3\1/2\ feet.
[GRAPHIC] [TIFF OMITTED] TC30OC91.021

Unsupported Vertically Sided Lower Portion--Maximum 8 Feet in Depth

All excavations more than 8 feet but not more than 12 feet in depth which unsupported vertically sided lower portions shall have a maximum allowable slope of 1:1 and a maximum vertical side of 3\1/2\ feet.
[GRAPHIC] [TIFF OMITTED] TC30OC91.022

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Unsupported Vertically Sided Lower Portion--Maximum 12 Feet in Depth

All excavations 20 feet or less in depth which have vertically sided lower portions that are supported or shielded shall have a maximum allowable slope of \3/4\:1. The support or shield system must extend at least 18 inches above the top of the vertical side.
[GRAPHIC] [TIFF OMITTED] TC30OC91.023

Supported or Shielded Vertically Sided Lower Portion

4. All other simple slope, compound slope, and vertically sided lower portion excavations shall be in accordance with the other options permitted under Sec. 1926.**652**(b).

B-1.2 Excavations Made in Type B Soil

1. All simple slope excavations 20 feet or less in depth shall have a maximum allowable slope of 1:1.
[GRAPHIC] [TIFF OMITTED] TC30OC91.024

Simple Slope

2. All benched excavations 20 feet or less in depth shall have a maximum allowable slope of 1:1 and maximum bench dimensions as follows:

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[GRAPHIC] [TIFF OMITTED] TC30OC91.025

Single Bench
[GRAPHIC] [TIFF OMITTED] TC30OC91.026

Multiple Bench

3. All excavations 20 feet or less in depth which have vertically sided lower portions shall be shielded or supported to a height at least 18 inches above the top of the vertical side. All such excavations shall have a maximum allowable slope of 1:1.
[GRAPHIC] [TIFF OMITTED] TC30OC91.027

Vertically Sided Lower Portion

4. All other sloped excavations shall be in accordance with the other options permitted in Sec. 1926.**652**(b).

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B-1.3 Excavations Made in Type C Soil

1. All simple slope excavations 20 feet or less in depth shall have a maximum allowable slope of 1\1/2\:1.
[GRAPHIC] [TIFF OMITTED] TC30OC91.028

Simple Slope

2. All excavations 20 feet or less in depth which have vertically sided lower portions shall be shielded or supported to a height at least 18 inches above the top of the vertical side. All such excavations shall have a maximum allowable slope of 1\1/2\:1.
[GRAPHIC] [TIFF OMITTED] TC30OC91.029

Vertical Sided Lower Portion

3. All other sloped excavations shall be in accordance with the other options permitted in Sec. 1926.**652**(b).

B-1.4 Excavations Made in Layered Soils

1. All excavations 20 feet or less in depth made in layered soils shall have a maximum allowable slope for each layer as set forth below.

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[GRAPHIC] [TIFF OMITTED] TC30OC91.030

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[GRAPHIC] [TIFF OMITTED] TC30OC91.031

2. All other sloped excavations shall be in accordance with the other options permitted in Sec. 1926.**652**(b).

Appendix C to Subpart P of Part 1926--Timber Shoring for Trenches

(a) Scope. This appendix contains information that can be used timber shoring is provided as a method of protection from cave-ins in trenches that do not exceed 20 feet (6.1 m) in depth. This appendix must be used when design of timber shoring protective systems is to be performed in accordance with Sec. 1926.**652**(c)(1). Other timber shoring configurations; other systems of support such as hydraulic and pneumatic systems; and other protective systems such as sloping, benching, shielding, and freezing systems must be designed in accordance with the requirements set forth in Sec. 1926.**652**(b) and Sec. 1926.**652**(c).

(b) Soil Classification. In order to use the data presented in this appendix, the soil type or types in which the excavation is made must first be determined using the soil classification method set forth in appendix A of subpart P of this part.

(c) Presentation of Information. Information is presented in several forms as follows:

(1) Information is presented in tabular form in Tables C-1.1, C-1.2,

and C-1.3, and Tables C-2.1, C-2.2 and C-2.3 following paragraph (g) of the appendix. Each table presents the minimum sizes of timber members to use in a shoring system, and each table contains data only for the particular soil type in which the excavation or portion of

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the excavation is made. The data are arranged to allow the user the flexibility to select from among several acceptable configurations of members based on varying the horizontal spacing of the crossbraces. Stable rock is exempt from shoring requirements and therefore, no data are presented for this condition.

(2) Information concerning the basis of the tabular data and the limitations of the data is presented in paragraph (d) of this appendix, and on the tables themselves.

(3) Information explaining the use of the tabular data is presented in paragraph (e) of this appendix.

(4) Information illustrating the use of the tabular data is presented in paragraph (f) of this appendix.

(5) Miscellaneous notations regarding Tables C-1.1 through C-1.3 and Tables C-2.1 through C-2.3 are presented in paragraph (g) of this Appendix.

(d) Basis and limitations of the data.--(1) Dimensions of timber members. (i) The sizes of the timber members listed in Tables C-1.1 through C-1.3 are taken from the National Bureau of Standards (NBS) report, ``Recommended Technical Provisions for Construction Practice in Shoring and Sloping of Trenches and Excavations.'' In addition, where NBS did not recommend specific sizes of members, member sizes are based on an analysis of the sizes required for use by existing codes and on empirical practice.

(ii) The required dimensions of the members listed in Tables C-1.1 through C-1.3 refer to actual dimensions and not nominal dimensions of the timber. Employers wanting to use nominal size shoring are directed to Tables C-2.1 through C-2.3, or have this choice under Sec. 1926.**652**(c)(3), and are referred to The Corps of Engineers, The Bureau of Reclamation or data from other acceptable sources.

(2) Limitation of application. (i) It is not intended that the timber shoring specification apply to every situation that may be experienced in the field. These data were developed to apply to the situations that are most commonly experienced in current trenching practice. Shoring systems for use in situations that are not covered by the data in this appendix must be designed as specified in Sec. 1926.**652**(c).

(ii) When any of the following conditions are present, the members specified in the tables are not considered adequate. Either an alternate timber shoring system must be designed or another type of protective system designed in accordance with Sec. 1926.**652**.

(A) When loads imposed by structures or by stored material adjacent to the trench weigh in excess of the load imposed by a two-foot soil surcharge. The term ``adjacent'' as used here means the area within a horizontal distance from the edge of the trench equal to the depth of the trench.

(B) When vertical loads imposed on cross braces exceed a 240-pound gravity load distributed on a one-foot section of the center of the crossbrace.

(C) When surcharge loads are present from equipment weighing in excess of 20,000 pounds.

(D) When only the lower portion of a trench is shored and the remaining portion of the trench is sloped or benched unless: The sloped portion is sloped at an angle less steep than three horizontal to one vertical; or the members are selected from the tables for use at a depth which is determined from the top of the overall trench, and not from the toe of the sloped portion.

(e) Use of Tables. The members of the shoring system that are to be selected using this information are the cross braces, the uprights, and the wales, where wales are required. Minimum sizes of members are specified for use in different types of soil. There are six tables of information, two for each soil type. The soil type must first be determined in accordance with the soil classification system described in appendix A to subpart P of part 1926. Using the appropriate table, the selection of the size and spacing of the members is then made. The selection is based on the depth and width of the trench where the members are to be installed and, in most instances, the selection is also based on the horizontal spacing of the crossbraces. Instances where a choice of horizontal spacing of crossbracing is available, the horizontal spacing of the crossbraces must be chosen by the user before the size of any member can be determined. When the soil type, the width and depth of the trench, and the horizontal spacing of the crossbraces are known, the size and vertical spacing of the crossbraces, the size and vertical spacing of the wales, and the size and horizontal spacing of the uprights can be read from the appropriate table.

(f) Examples to Illustrate the Use of Tables C-1.1 through C-1.3.

(1) Example 1.

A trench dug in Type A soil is 13 feet deep and five feet wide.

From Table C-1.1, for acceptable arrangements of timber can be used.

Arrangement #B1

Space 4x4 crossbraces at six feet horizontally and four feet vertically.

Wales are not required.

Space 3x8 uprights at six feet horizontally. This arrangement is commonly called ``skip shoring.''

Arrangement #B2

Space 4x6 crossbraces at eight feet horizontally and four feet vertically.

Space 8x8 wales at four feet vertically.

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Space 2x6 uprights at four feet horizontally.

Arrangement #B3

Space 6x6 crossbraces at 10 feet horizontally and four feet vertically.

Space 8x10 wales at four feet vertically.

Space 2x6 uprights at five feet horizontally.

Arrangement #B4

Space 6x6 crossbraces at 12 feet horizontally and four feet vertically.

Space 10x10 wales at four feet vertically.

Spaces 3x8 uprights at six feet horizontally.

(2) Example 2.

A trench dug in Type B soil in 13 feet deep and five feet wide. From Table C-1.2 three acceptable arrangements of members are listed.

Arrangement #B1

Space 6x6 crossbraces at six feet horizontally and five feet vertically.
Space 8x8 wales at five feet vertically.
Space 2x6 uprights at two feet horizontally.

Arrangement #B2

Space 6x8 crossbraces at eight feet horizontally and five feet vertically.
Space 10x10 wales at five feet vertically.
Space 2x6 uprights at two feet horizontally.

Arrangement #B3

Space 8x8 crossbraces at 10 feet horizontally and five feet vertically.
Space 10x12 wales at five feet vertically.
Space 2x6 uprights at two feet vertically.
(3) Example 3.
A trench dug in Type C soil is 13 feet deep and five feet wide.
From Table C-1.3 two acceptable arrangements of members can be used.

Arrangement #B1

Space 8x8 crossbraces at six feet horizontally and five feet vertically.
Space 10x12 wales at five feet vertically.
Position 2x6 uprights as closely together as possible.
If water must be retained use special tongue and groove uprights to form tight sheeting.

Arrangement #B2

Space 8x10 crossbraces at eight feet horizontally and five feet vertically.
Space 12x12 wales at five feet vertically.
Position 2x6 uprights in a close sheeting configuration unless water pressure must be resisted. Tight sheeting must be used where water must be retained.
(4) Example 4.
A trench dug in Type C soil is 20 feet deep and 11 feet wide. The size and spacing of members for the section of trench that is over 15 feet in depth is determined using Table C-1.3. Only one arrangement of members is provided.
Space 8x10 crossbraces at six feet horizontally and five feet vertically.
Space 12x12 wales at five feet vertically.
Use 3x6 tight sheeting.
Use of Tables C-2.1 through C-2.3 would follow the same procedures.
(g) Notes for all Tables.
1. Member sizes at spacings other than indicated are to be determined as specified in Sec. 1926.**652**(c), ``Design of Protective Systems.''
2. When conditions are saturated or submerged use Tight Sheeting. Tight Sheeting refers to the use of specially-edged timber planks (e.g., tongue and groove) at least three inches thick, steel sheet piling, or similar construction that when driven or placed in position provide a

tight wall to resist the lateral pressure of water and to prevent the loss of backfill material. Close Sheeting refers to the placement of planks side-by-side allowing as little space as possible between them.

3. All spacing indicated is measured center to center.

4. Wales to be installed with greater dimension horizontal.

5. If the vertical distance from the center of the lowest crossbrace to the bottom of the trench exceeds two and one-half feet, uprights shall be firmly embedded or a mudsill shall be used. Where uprights are embedded, the vertical distance from the center of the lowest crossbrace to the bottom of the trench shall not exceed 36 inches. When mudsills are used, the vertical distance shall not exceed 42 inches. Mudsills are wales that are installed at the toe of the trench side.

6. Trench jacks may be used in lieu of or in combination with timber crossbraces.

7. Placement of crossbraces. When the vertical spacing of crossbraces is four feet, place the top crossbrace no more than two feet below the top of the trench. When the vertical spacing of crossbraces is five feet, place the top crossbrace no more than 2.5 feet below the top of the trench.

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[GRAPHIC] [TIFF OMITTED] TC30OC91.032

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[GRAPHIC] [TIFF OMITTED] TC30OC91.033

[[Page 395]]

[GRAPHIC] [TIFF OMITTED] TC30OC91.034

[[Page 396]]

[GRAPHIC] [TIFF OMITTED] TC30OC91.035

[[Page 397]]

[GRAPHIC] [TIFF OMITTED] TC30OC91.036

[[Page 398]]

[GRAPHIC] [TIFF OMITTED] TC30OC91.037

Appendix D to Subpart P of Part 1926--Aluminum Hydraulic Shoring for Trenches

(a) Scope. This appendix contains information that can be used when aluminum hydraulic shoring is provided as a method of protection against cave-ins in trenches that do not exceed 20 feet (6.1m) in depth. This appendix must be used when design of the aluminum hydraulic protective system cannot be performed in accordance with Sec. 1926.**652**(c)(2).

(b) Soil Classification. In order to use data presented in this appendix, the soil type or types in which the excavation is made must

first be determined using the soil classification method set forth in appendix A of subpart P of part 1926.

(c) Presentation of Information. Information is presented in several forms as follows:

(1) Information is presented in tabular form in Tables D-1.1, D-1.2, D-1.3 and E-1.4. Each table presents the maximum vertical and horizontal spacings that may be used with various aluminum member sizes and various hydraulic cylinder sizes. Each table contains data only for the particular soil type in which the excavation or portion of the excavation is made. Tables D-1.1 and D-1.2 are for vertical shores in Types A and B soil. Tables D-1.3 and D1.4 are for horizontal waler systems in Types B and C soil.

(2) Information concerning the basis of the tabular data and the limitations of the data is presented in paragraph (d) of this appendix.

(3) Information explaining the use of the tabular data is presented in paragraph (e) of this appendix.

(4) Information illustrating the use of the tabular data is presented in paragraph (f) of this appendix.

(5) Miscellaneous notations (footnotes) regarding Table D-1.1 through D-1.4 are presented in paragraph (g) of this appendix.

(6) Figures, illustrating typical installations of hydraulic shoring, are included just prior to the Tables. The illustrations page is entitled ``Aluminum Hydraulic Shoring; Typical Installations.''

(d) Basis and limitations of the data.

(1) Vertical shore rails and horizontal wales are those that meet the Section Modulus requirements in the D-1 Tables. Aluminum material is 6061-T6 or material of equivalent strength and properties.

(2) Hydraulic cylinders specifications. (i) 2-inch cylinders shall be a minimum 2-inch inside diameter with a minimum safe working capacity of no less than 18,000 pounds axial compressive load at maximum extension. Maximum extension is to include full range of cylinder extensions as recommended by product manufaturer.

(ii) 3-inch cylinders shall be a minimum 3-inch inside diameter with a safe working capacity of not less than 30,000 pounds axial compressive load at extensions as recommended by product manufacturer.

(3) Limitation of application.

(i) It is not intended that the aluminum hydraulic specification apply to every situation that may be experienced in the field. These data were developed to apply to the situations that are most commonly experienced in current trenching practice. Shoring systems for use in situations that are not covered by the data in this appendix must be otherwise designed as specified in Sec. 1926.**652**(c).

(ii) When any of the following conditions are present, the members specified in the Tables are not considered adequate. In this case, an alternative aluminum hydraulic shoring system or other type of protective system must be designed in accordance with Sec. 1926.**652**.

(A) When vertical loads imposed on cross braces exceed a 100 Pound gravity load distributed on a one foot section of the center of the hydraulic cylinder.

(B) When surcharge loads are present from equipment weighing in excess of 20,000 pounds.

(C) When only the lower portion or a trench is shored and the remaining portion of the trench is sloped or benched unless: The sloped portion is sloped at an angle less steep than three horizontal to one vertical; or the members are selected from the tables for use at a depth which is determined from the top of the overall trench, and not from the toe of the sloped portion.

(e) Use of Tables D-1.1, D-1.2, D-1.3 and D-1.4. The members of the shoring system that are to be selected using this information are the hydraulic cylinders, and either the vertical shores or the horizontal wales. When a waler system is used the vertical timber sheeting to be used is also selected from these tables. The Tables D-1.1 and D-1.2 for vertical shores are used in Type A and B soils that do not require sheeting. Type B soils that may require sheeting, and Type C soils that always require sheeting are found in the horizontal wale Tables D-1.3 and D-1.4. The soil type must first be determined in accordance with the soil classification system described in appendix A to subpart P of part 1926. Using the appropriate table, the selection of the size and spacing of the members is made. The selection is based on the depth and width of the trench where the members are to be installed. In these tables the vertical spacing is held constant at four feet on center. The tables show the maximum horizontal spacing of cylinders allowed for each size of wale in the waler system tables, and in the vertical shore tables, the hydraulic cylinder horizontal spacing is the same as the vertical shore spacing.

(f) Example to Illustrate the Use of the Tables:

(1) Example 1:

A trench dug in Type A soil is 6 feet deep and 3 feet wide. From Table D-1.1: Find vertical shores and 2 inch diameter cylinders spaced 8 feet on center (o.c.) horizontally and 4 feet on center (o.c.) vertically. (See Figures 1 & 3 for typical installations.)

(2) Example 2:

A trench is dug in Type B soil that does not require sheeting, 13 feet deep and 5 feet wide. From Table D-1.2: Find vertical shores and 2 inch diameter cylinders spaced 6.5 feet o.c. horizontally and 4 feet o.c. vertically. (See Figures 1 & 3 for typical installations.)

(3) A trench is dug in Type B soil that does not require sheeting, but does experience some minor raveling of the trench face. The

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trench is 16 feet deep and 9 feet wide. From Table D-1.2: Find vertical shores and 2 inch diameter cylinder (with special oversleeves as designated by footnote <greek-i>B2) spaced 5.5 feet o.c. horizontally and 4 feet o.c. vertically, plywood (per footnote (g)(7) to the D-1 Table) should be used behind the shores. (See Figures 2 & 3 for typical installations.)

(4) Example 4: A trench is dug in previously disturbed Type B soil, with characteristics of a Type C soil, and will require sheeting. The trench is 18 feet deep and 12 feet wide. 8 foot horizontal spacing between cylinders is desired for working space. From Table D-1.3: Find horizontal wale with a section modulus of 14.0 spaced at 4 feet o.c. vertically and 3 inch diameter cylinder spaced at 9 feet maximum o.c. horizontally. 3x12 timber sheeting is required at close spacing vertically. (See Figure 4 for typical installation.)

(5) Example 5: A trench is dug in Type C soil, 9 feet deep and 4 feet wide. Horizontal cylinder spacing in excess of 6 feet is desired for working space. From Table D-1.4: Find horizontal wale with a section modulus of 7.0 and 2 inch diameter cylinders spaced at 6.5 feet o.c. horizontally. Or, find horizontal wale with a 14.0 section modulus and 3 inch diameter cylinder spaced at 10 feet o.c. horizontally. Both wales are spaced 4 feet o.c. vertically. 3x12 timber sheeting is required at close spacing vertically. (See Figure 4 for typical installation.)

(g) Footnotes, and general notes, for Tables D-1.1, D-1.2, D-1.3, and D-1.4.

(1) For applications other than those listed in the tables, refer to Sec. 1926.**652**(c)(2) for use of manufacturer's tabulated data. For trench depths in excess of 20 feet, refer to Sec. 1926.**652**(c)(2) and Sec. 1926.**652**(c)(3).

(2) 2 inch diameter cylinders, at this width, shall have structural steel tube (3.5x3.5x0.1875) oversleeves, or structural oversleeves of manufacturer's specification, extending the full, collapsed length.

(3) Hydraulic cylinders capacities. (i) 2 inch cylinders shall be a minimum 2-inch inside diameter with a safe working capacity of not less than 18,000 pounds axial compressive load at maximum extension. Maximum extension is to include full range of cylinder extensions as recommended by product manufacturer.

(ii) 3-inch cylinders shall be a minimum 3-inch inside diameter with a safe work capacity of not less than 30,000 pounds axial compressive load at maximum extension. Maximum extension is to include full range of cylinder extensions as recommended by product manufacturer.

(4) All spacing indicated is measured center to center.

(5) Vertical shoring rails shall have a minimum section modulus of 0.40 inch.

(6) When vertical shores are used, there must be a minimum of three shores spaced equally, horizontally, in a group.

(7) Plywood shall be 1.125 in. thick softwood or 0.75 inch. thick, 14 ply, arctic white birch (Finland form). Please note that plywood is not intended as a structural member, but only for prevention of local raveling (sloughing of the trench face) between shores.

(8) See appendix C for timber specifications.

(9) Wales are calculated for simple span conditions.

(10) See appendix D, item (d), for basis and limitations of the data.

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[GRAPHIC] [TIFF OMITTED] TC30OC91.038

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[GRAPHIC] [TIFF OMITTED] TC30OC91.039

[[Page 403]]

[GRAPHIC] [TIFF OMITTED] TC30OC91.040

[[Page 404]]

[GRAPHIC] [TIFF OMITTED] TC30OC91.041

[[Page 405]]

[GRAPHIC] [TIFF OMITTED] TC30OC91.042

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Appendix E to Subpart P of Part 1926--Alternatives to Timber Shoring

[GRAPHIC] [TIFF OMITTED] TC30OC91.043

[[Page 407]]

[GRAPHIC] [TIFF OMITTED] TC30OC91.044

Appendix F to Subpart P of Part 1926--Selection of Protective Systems

The following figures are a graphic summary of the requirements contained in subpart P for excavations 20 feet or less in depth. Protective systems for use in excavations more than 20 feet in depth must be designed by a registered professional engineer in accordance with Sec. 1926.**652** (b) and (c).

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[GRAPHIC] [TIFF OMITTED] TC30OC91.045

[[Page 409]]

[GRAPHIC] [TIFF OMITTED] TC30OC91.046

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[GRAPHIC] [TIFF OMITTED] TC30OC91.047

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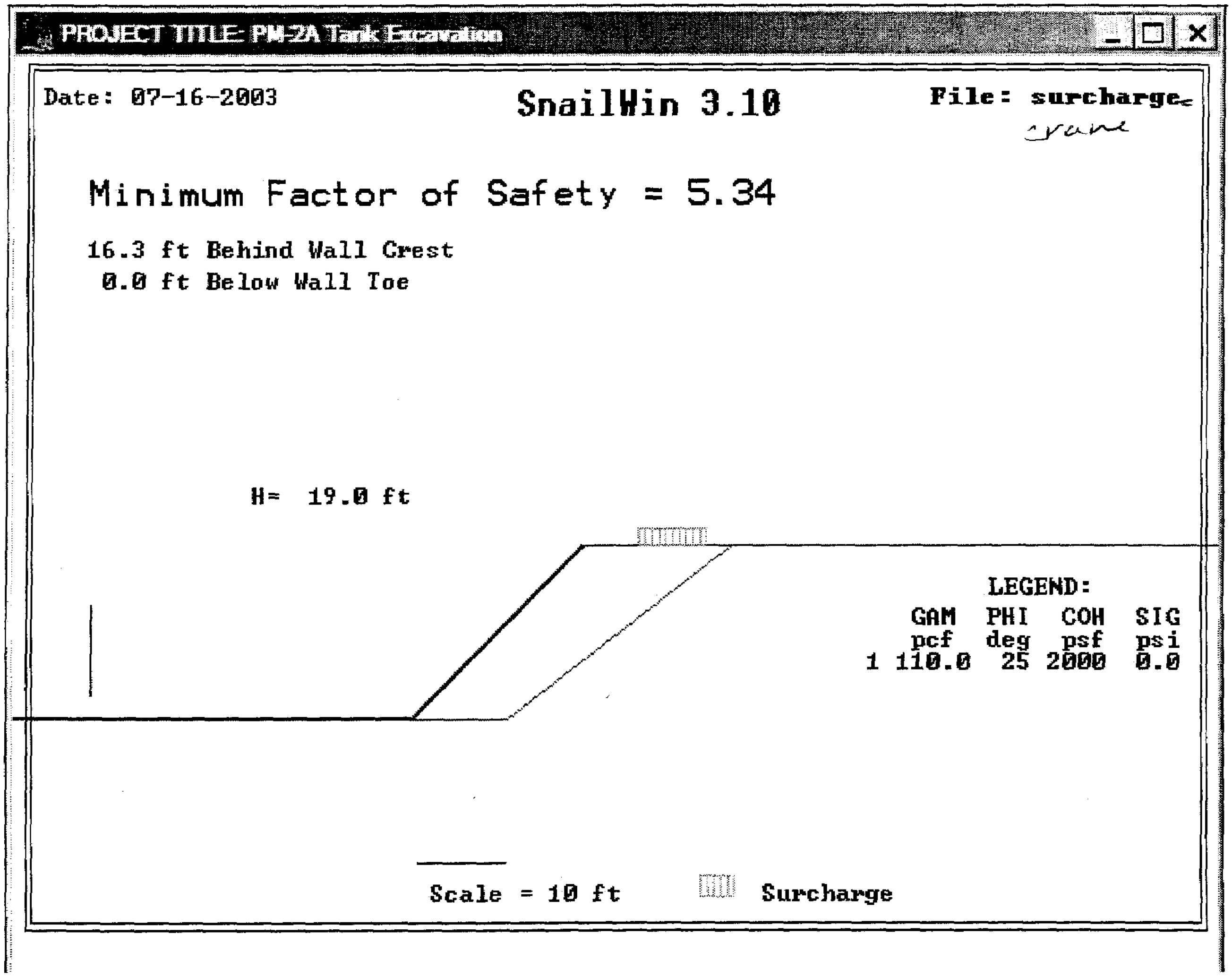
Appendix F: SNAILZ Model


PROJECT TITLE: PM-2A Tank Excavation
Date: 07-16-2003
SnailWin 3.10
File: surcharge
crane
Minimum Factor of Safety = 5.34
16.3 ft Behind Wall Crest
0.0 ft Below Wall Toe
H= 19.0 ft
LEGEND:
GAM PHI COH SIG
pcf deg psf psi
1 110.0 25 2000 0.0
Scale = 10 ft
Surcharge

```
************************************************
*    CALIFORNIA DEPARTMENT OF TRANSPORTATION    *
*          ENGINEERING SERVICE CENTER           *
*     DIVISION OF MATERIALS AND FOUNDATIONS     *
*   Office of Roadway Geotechnical Engineering  *
*    Date: 07-16-2003          Time: 16:35:02   *
************************************************
```

Project Identification - PM-2A Tank Excavation

--------- WALL GEOMETRY ---------

```
Vertical Wall Height           =  19.0 ft
Wall Batter                    =  45.0 degree
                                 Angle   Length
                                 (Deg)   (Feet)
First Slope from Wallcrest.    =   0.0     50.0
Second Slope from 1st slope.   =   0.0      0.0
Third Slope from 2nd slope.    =   0.0      0.0
Fourth Slope from 3rd slope.   =   0.0      0.0
Fifth Slope from 3rd slope.    =   0.0      0.0
Sixth Slope from 3rd slope.    =   0.0      0.0
Seventh Slope Angle.           =   0.0
```

--------- SLOPE BELOW THE WALL ---------

```
First Slope Angle below Toe.       =   0.0 degrees
First Slope Distance from Toe.     =  20.0 ft
Second Slope Angle.                =   0.0 degrees
Second Slope Distance from Toe.    =   0.0 ft
Vertical Depth of Search.          =   7.5 ft
Number of Searches below wall Toe. =   5
```

--------- SURCHARGE ---------

THE SURCHARGES IMPOSED ON THE SYSTEM ARE:

```
Begin Surcharge - Distance from toe =     25.0 ft
End Surcharge - Distance from toe   =     33.0 ft
Loading Intensity - Begin           =   1125.0 psf/ft
Loading Intensity - End             =   1125.0 psf/ft
```

--------- OPTION #1 ---------

Ultimate Punching shear, Bond & Yield Stress are used.

--------- SOIL PARAMETERS ---------

Soil Layer	Unit Weight (Pcf)	Friction Angle (Degree)	Cohesion Intercept (Psf)	Bond* Stress (Psi)	Coordinates of Boundary XS1 (ft)	YS1 (ft)	XS2 (ft)	YS2 (ft)
1	110.0	25.0	2000.0	0.0	0.0	0.0	0.0	0.0

* Ultimate bond Stress values also depend on BSF (Bond Stress Factor.)

--------- WATER SURFACE ---------

NO Water Table defined for this problem.

--------- SEARCH LIMIT ---------

The Search Limit is from 1.0 to 50.0 ft

You have chosen NOT TO LIMIT the search of failure planes to specific nodes.

--------- REINFORCEMENT PARAMETERS ---------

Number of Reinforcement Levels	=	0
Horizontal Spacing	=	5.0 ft
Diameter of Reinforcement Element	=	0.000 in
Yield Stress of Reinforcement	=	0.0 ksi
Diameter of Grouted Hole	=	0.0 in
Punching Shear	=	0.0 kips

--------- (For ALL Levels) ---------

Reinforcement Lengths	=	0.0 ft
Reinforcement Inclination	=	0.0 degrees
Vertical Spacing to First Level	=	0.0 ft
Vertical Spacing to Remaining Levels	=	0.0 ft

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
Toe	5.34	35.3	0.0	10.6	37.6	31.2

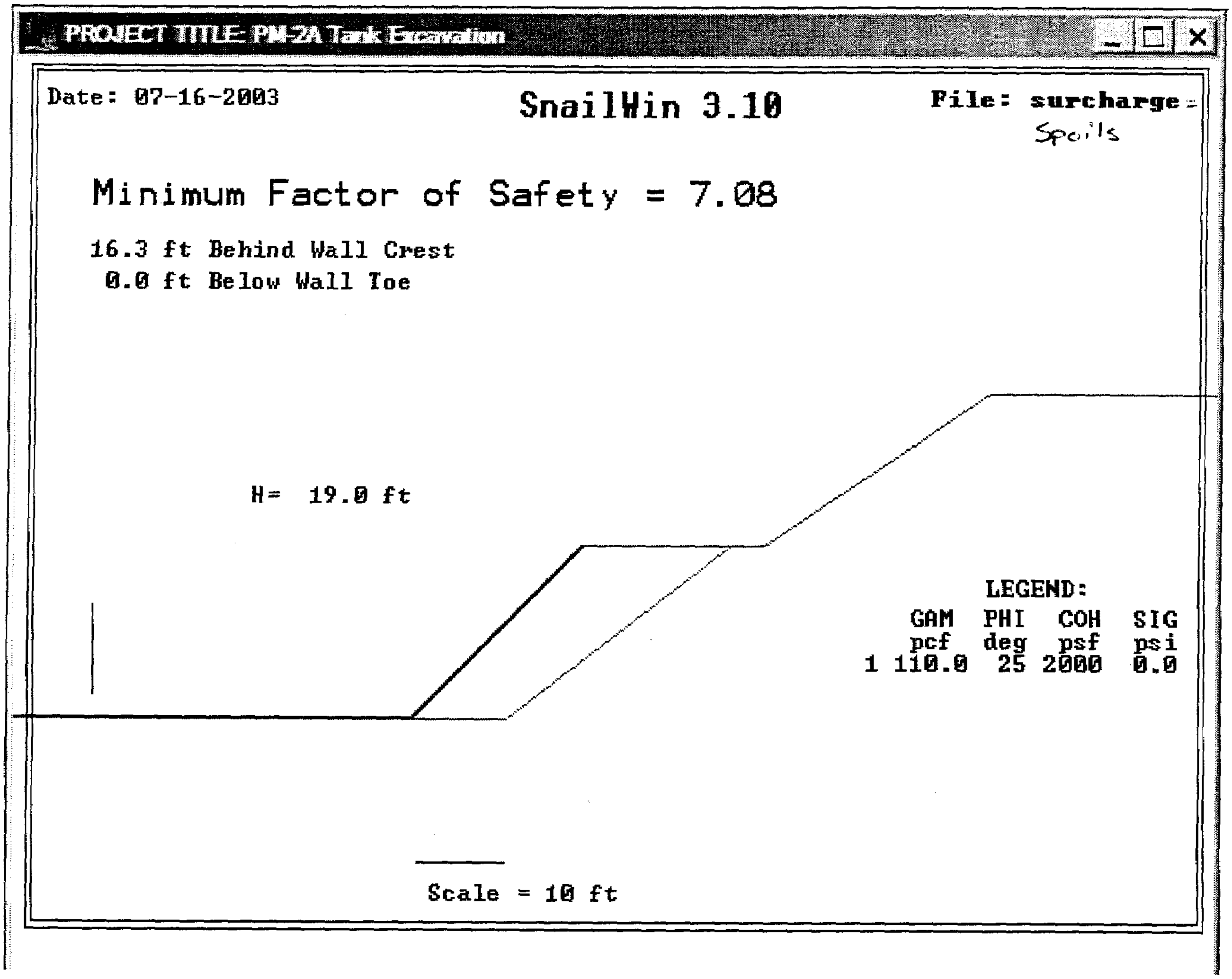
DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
1.50	5.62	35.3	8.3	14.3	41.1	28.1

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
3.00	5.84	35.3	0.0	14.1	46.1	30.5

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
4.50	6.16	35.3	0.0	10.6	43.6	34.1

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
6.00	6.38	40.2	0.0	12.1	41.6	37.6

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
7.50	6.56	40.2	0.0	12.1	43.3	38.7


PROJECT TITLE: PM-2A Tank Excavation
Date: 07-16-2003
SnailWin 3.10
File: surcharge
Spoils
Minimum Factor of Safety = 7.08
16.3 ft Behind Wall Crest
0.0 ft Below Wall Toe
H= 19.0 ft
LEGEND:
GAM pcf PHI deg COH psf SIG psi
1 110.0 25 2000 0.0
Scale = 10 ft

```
************************************************
*    CALIFORNIA DEPARTMENT OF TRANSPORTATION    *
*          ENGINEERING SERVICE CENTER           *
*     DIVISION OF MATERIALS AND FOUNDATIONS     *
*   Office of Roadway Geotechnical Engineering  *
*    Date: 07-16-2003          Time: 16:00:02   *
************************************************
```

Surcharge = spoils

Project Identification - PM-2A Tank Excavation

--------- WALL GEOMETRY ---------

		Angle (Deg)	Length (Feet)
Vertical Wall Height	=	19.0 ft	
Wall Batter	=	45.0 degree	
First Slope from Wallcrest.	=	0.0	20.0
Second Slope from 1st slope.	=	34.0	30.0
Third Slope from 2nd slope.	=	0.0	0.0
Fourth Slope from 3rd slope.	=	0.0	0.0
Fifth Slope from 3rd slope.	=	0.0	0.0
Sixth Slope from 3rd slope.	=	0.0	0.0
Seventh Slope Angle.	=	0.0	

--------- SLOPE BELOW THE WALL ---------

First Slope Angle below Toe.	=	0.0 degrees
First Slope Distance from Toe.	=	20.0 ft
Second Slope Angle.	=	0.0 degrees
Second Slope Distance from Toe.	=	0.0 ft
Vertical Depth of Search.	=	7.5 ft
Number of Searches below wall Toe.	=	5

--------- SURCHARGE ---------

There is NO SURCHARGE imposed on the system.

--------- OPTION #1 ---------

Ultimate Punching shear, Bond & Yield Stress are used.

--------- SOIL PARAMETERS ---------

Soil Layer	Unit Weight (Pcf)	Friction Angle (Degree)	Cohesion Intercept (Psf)	Bond* Stress (Psi)	Coordinates of Boundary XS1 (ft)	YS1 (ft)	XS2 (ft)	YS2 (ft)
1	110.0	25.0	2000.0	0.0	0.0	0.0	0.0	0.0

* Ultimate bond Stress values also depend on BSF (Bond Stress Factor.)

--------- WATER SURFACE ---------

NO Water Table defined for this problem.

--------- SEARCH LIMIT ---------

The Search Limit is from 1.0 to 50.0 ft

You have chosen NOT TO LIMIT the search of failure planes to specific nodes.

--------- REINFORCEMENT PARAMETERS ---------

Number of Reinforcement Levels	= 0
Horizontal Spacing	= 5.0 ft
Diameter of Reinforcement Element	= 0.000 in
Yield Stress of Reinforcement	= 0.0 ksi
Diameter of Grouted Hole	= 0.0 in
Punching Shear	= 0.0 kips

--------- (For ALL Levels) ---------

Reinforcement Lengths	= 0.0 ft
Reinforcement Inclination	= 0.0 degrees
Vertical Spacing to First Level	= 0.0 ft
Vertical Spacing to Remaining Levels	= 0.0 ft

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
Toe	7.08	35.3	0.0	10.6	37.6	31.2

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
1.50	7.43	35.3	0.0	10.6	39.7	32.1

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
3.00	7.67	40.2	0.0	12.1	39.0	36.2

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
4.50	7.84	40.2	0.0	12.1	40.8	37.2

DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
6.00	7.99	40.2	0.0	12.1	42.5	38.2

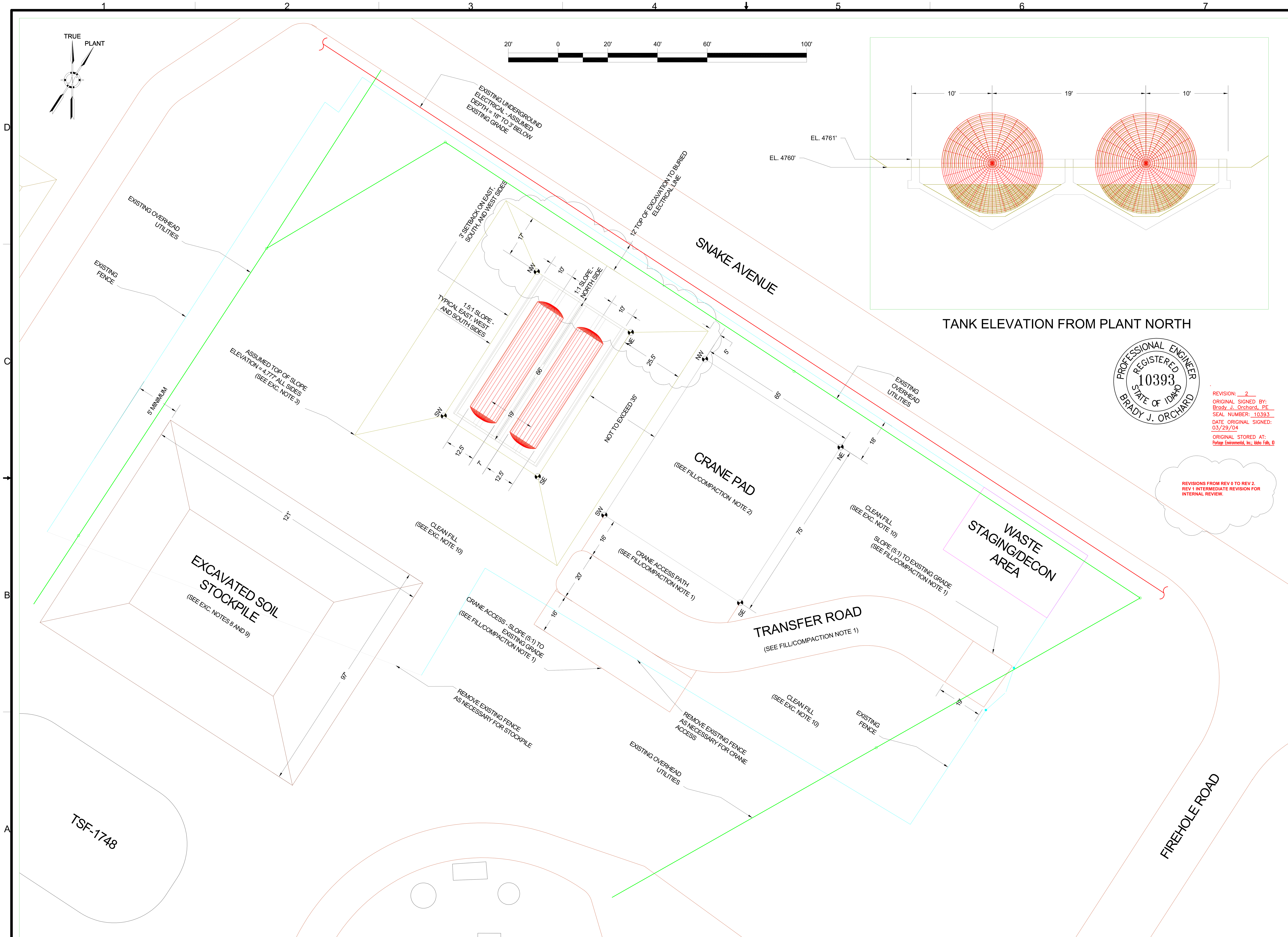
DEPTH BELOW WALL TOE (ft)	MINIMUM SAFETY FACTOR	DISTANCE BEHIND WALL TOE (ft)	LOWER FAILURE PLANE ANGLE (deg)	LOWER FAILURE PLANE LENGTH (ft)	UPPER FAILURE PLANE ANGLE (deg)	UPPER FAILURE PLANE LENGTH (ft)
7.50	8.13	40.2	0.0	12.1	44.1	39.2

Appendix B

Preliminary Design Information

Manitowoc 2250 Crane with Maxer
Total Weight and Ground Pressure Estimates for 200,000 lb Load

Component/Description	Quantity	Component Weight (lb)	Subtotal Weight (lb)
Basic Crane Parts:			372,310
Upperworks	1	85,020	
Carbody, Etc.	1	64,350	
Crawlers	2	53,820	
Basic Counterweights	1	169,120	
No. 44 Mast (130 ft)			60,920
Mast Top	1	28,420	
40-ft Mast Insert	1	6,500	
10-ft Mast Insert	1	2,240	
Mast Butt	1	23,760	
No. 79 Boom (140 ft)			60,514
Boom Top	1	16,248	
25-ft Transition Insert	1	8,053	
40-ft Boom Insert	1	12,060	
40-ft Boom Insert w/Sheaves	1	11,988	
Boom Butt	1	12,165	
Balanced Loads			400,000
Lift Load	1	200,000	
Maxer Counterweight Resistance	1	200,000	
TOTAL GROUND FORCE (lb)			**893,744**
Track Surface Area (sq ft)			214
Width		4.00	
Length Contacting Ground		26.75	
Number of Tracks		2	
TOTAL GROUND PRESSURE (psf)			**4,176**



REVISIONS

REV	DESCRIPTION	EFFECTIVE DATE
1	1:1 SLOPE NORTH SIDE; REVISED FILL SPECIFICATIONS	03/25/04
2	ADDED CONDITIONS FOR 1:1 SLOPE NORTH SIDE	03/29/04

EXCAVATION NOTES

1. COMPLETE WORK OUTLINED IN SHEETS C-1 AND C-2 (RD/RA WORK PLAN [DOE/ID-11073, REVISION 0, DECEMBER 2003] DRAWING NO. 10355 AND 10356) . ADDITIONAL 2 FT OF SOIL TO BE REMOVED FROM SURFACE OF ENTIRE SITE PRIOR TO TANK EXCAVATION ACTIVITIES **EXCEPT AS IDENTIFIED BELOW**. SITE ELEVATION AT BEGINNING OF TANK EXCAVATION TO BE APPROXIMATELY 4777 AT NORTH END OF TANKS (CURRENT ELEVATION APPROXIMATEY 4779).
   - **18-IN. SETBACK FROM EXISTING EXCAVATION SIDES TO BE MAINTAINED ON NORTH AND WEST SIDES OF SITE.**
   - **7-FT RADIAL SETBACK TO BE MAINTAINED AROUND ALL POWER POLES AND GUY WIRES WITHIN SITE BOUNDARIES.**

2. LOCATION OF TANK EXCAVATION FOOTPRINT, SOIL STOCKPILE LOCATION, AND WASTE STAGING/DECON AREA SHOWN ON SHEETS C-3 AND C-5 (RD/RA WORK PLAN [DOE/ID-11073, REVISION 0, DECEMBER 2003] DRAWING NO. 10357 AND 10359) SUPERCEDED BY THIS DRAWING. NO ON-SITE TRAILER PAD TO BE CONSTRUCTED.

3. FINISH ELEVATIONS AND EXCAVATION CORNER COORDINATES ARE APPROXIMATE AND BASED ON THE BEST INFORMATION AVAILABLE AT THE TIME OF PREPARATION. ACUTAL EXCAVATION CORNER LOCATION AND FOOTPRINT WILL BE GOVERNED BY THE OFFSETS IDENTIFIED ON THIS DRAWING FOR DISTANCE FROM TANKS AS EXCAVATED TO TOE OF SLOPE AND EXCAVATION SIDE SLOPES. ACTUAL EXCAVATION DEPTH SHALL BE CONTROLLED BY THE ACTUAL ELEVATION OF THE TOP OF TANK CRADLES. FINAL TOE EXCAVATION ELEVATION SHALL BE 1 FT BELOW TOP OF TANK CRADLES AND AS SHOWN ON TANK ELEVATION INSET.

4. EXCAVATE AROUND TANKS TO ELEVATIONS AND LOCATIONS INDICATED ON EXCAVATION FOOTPRINT AND DESCRIBED IN NOTES 2 AND 3. EQUIPMENT SHALL BE CAPABLE OF EXCAVATING TO THE FULL REQUIRED DEPTH FROM THE GROUND SURFACE AND ALL EXCAVATION SHALL BE COMPLETED BY DIGGING DOWN FROM THE GROUND SURFACE ELEVATION ESTABLISHED IN THE PRELIMINARY SITE GRADING. EXCAVATING EQUIPMENT SHALL NOT BE PERMITTED INSIDE THE EXCAVATION EXCEPT AS REQUIRED TO PROVIDE A FINAL WORKING SURFACE AROUND THE TANKS.

5. EXCAVATE 1 FT BELOW TOP OF TANK CRADLE AS SHOWN TO ALLOW TANK ACCESS FOR INSTALLATION OF LIFTING DEVICES (TANK CRADLE DIMENSIONS AND CONFIGURAITONS ARE APPROXIMATE).

6. **EAST, WEST, AND SOUTH SIDE SLOPES** SHALL BE MAINTAINED AT A SLOPE OF 1.5:1. IF SOIL CONDITIONS ARE ENCOUNTERED THAT DO NOT MEET OSHA REQUIREMENTS FOR 1.5:1 SLOPES, THE SUBCONTRACTOR SHALL STOP WORK AND IMMEDIATELY NOTIFY THE CONTRACTOR. THE CONCERN WILL BE RESOLVED BEFORE WORK STARTS AGAIN.

7. **NORTH SIDE SLOPE SHALL BE MAINTAINED AT A SLOPE OF 1:1 BASED ON SLOPE STABILITY CALCULATIONS (PEI-EDF-1000, MARCH 2004).**
   - **SOIL MOISTURE CONDITIONS MUST BE CLOSELY MONITORED DURING FIELD WORK. WORK SHOULD NOT PROCEED IF EXPOSED SLOPE MATERIALS BECOME SATURATED.**
   - **EQUIPMENT SHOULD NOT TRAVEL ON THE GROUND SURFACE BETWEEN THE NORTH SIDE OF THE EXCAVATION AND SNAKE AVENUE.**

8. STOCKPILES OF EXCAVATED MATERIAL WILL BE LOCATED AS SHOWN. STOCKPILE SLOPES WILL BE MAINTAINED AT 1.5:1 AND BE LOCATED A MINIMUM OF 20 FT FROM THE EDGE OF THE EXCAVATION AND 5 FT FROM FENCES AND WORK AREAS. EXACT SOIL PILE LOCATION TO BE DETERMINED BY THESE OFFSETS IN THE FIELD. SOIL PILES SHALL COMPLY WITH CFR 29, OSHA REQUIREMENTS FOR SLOPE STABILITY. PILE HEIGHTS SHALL NOT EXCEED 20 FT.

9. SURVEY AND RECORD POSITIONS AND ELEVATIONS OF CRADLE CORNERS UPON EXPOSURE. UPON COMPLETION OF FINAL GRADING AND SEEDING OPERATIONS, PLACE 4"° BRASS CAP MARKERS AT EACH CORNER WITH THE FOLLOWING INFORMATION STAMPED INTO THE CAP (COORDINATES TO BE DETERMINED BASED ON SURVEY):
   OU1-10 PM-2A TANK CRADLE CORNER
   CRADLE CORNER ELEVATION: xxxx.x
   N xxxxxx.x
   E xxxxxx.x

10. FOLLOWING EXCAVATION, 6-IN. CLEAN FILL TO BE SPREAD AND LEVELED ON ENTIRE SITE TO MINIMIZE DECONTAMINATION EXCEPT WHERE FILL AND COMPACTION REQUIREMENTS SPECIFIED BELOW.

FILL/COMPACTION REQUIREMENTS

1. NEW TRANSFER ROAD, CRANE ACCESS PATH, AND RAMPS FOR CRANE/SITE ACCESS SHALL BE CONSTRUCTED FOLLOWING TANK EXCAVATION. **FILL MATERIALS SHALL BE PLACED IN LOOSE LIFTS NOT EXCEEDING 6-IN. IN THICKNESS, UNIFORMLY MOISTURE CONDITIONED AND COMPACTED TO A MINIMUM OF 90% OF MAXIMUM DRY DENSITY PER ASTM D 698, WITHIN 2% OF OPTIMUM MOISTURE CONTENT. ROAD AND CRANE ACCESS PATH SHALL BE LEVEL.** RAMPS SHALL BE SLOPED NO STEEPER THAN 5:1.

2. CRANE PAD SHALL BE CONSTRUCTED FOLLOWING TANK EXCAVATION. **FILL MATERIALS SHALL BE PLACED IN LOOSE LIFTS NOT EXCEEDING 6-IN. IN THICKNESS, UNIFORMLY MOISTURE CONDITIONED AND COMPACTED TO A MINIMUM OF 95% OF MAXIMUM DRY DENSITY PER ASTM D 698, WITHIN 2 % OF OPTIMUM MOISTURE CONTENT.** CRANE PAD SHALL BE LEVEL WITHIN 1/2 DEGREE IN ANY DIRECTION.

3. **ALL FILL MATERIALS FOR ROAD/RAMP/CRANE PAD TO CONSIST OF 3-IN MINUS OR 1.5-IN. MINUS PIT RUN GRAVEL OR 3/4-IN. CRUSH.**

4. DISTANCE BETWEEN EAST TOE OF EXCAVATION AND CRANE PAD NOT TO EXCEED 35 FT. IF SITE CONDITIONS NECESSITATE SLOPING SHALLOWER THAN SPECIFIED, THE SUBCONTRACTOR SHALL STOP WORK AND IMMEDIATELY NOTIFY THE CONTRACTOR.

PRELIMINARY CONTROL COORDINATES

TOE OF EXCAVATION SLOPE	CRANE PAD
NW (X,Y,Z) - 357375, 795676, 4760	NW (X,Y,Z) - 357443, 795641, 4777
SW (X,Y,Z) - 357338, 795619, 4760	SW (X,Y,Z) - 357402, 795579, 4777
SE (X,Y,Z) - 357375, 795594, 4760	SE (X,Y,Z) - 357457, 795543, 4777
NE (X,Y,Z) - 357403, 795649, 4760	NE (X,Y,Z) - 357497, 795606, 4777

EXCAVATION QUANTITIES

TANK EXCAVATION AND REPLACEMENT - **4,000 YD3**

REFERENCES

DOE-ID 2003, "REMEDIAL DESIGN/REMEDIAL ACTION WORK PLAN FOR GROUP 3, PM-2A TANKS AND BURN PITS FOR TEST AREA NORTH, WASTE AREA GROUP 1, OPERABLE UNIT 1-10," UNITED STATES DEPARTMENT OF ENERGY - IDAHO OPERATIONS OFFICE, DOE/ID-11073, REVISION 0, DECEMBER 2003.

DRAWING SUPERCEDES GROUP 3 RD/RA WORK PLAN (TAN, WAG1, OU 1-10) DRAWING NO. 10357 - 10359

Portage ENVIRONMENTAL
IDAHO OPERATIONS OFFICE
P.O. BOX 3726
IDAHO FALLS, ID 83403
PHONE: 208.528.6608
FAX: 208.523.8860

IDAHO COMPLETION PROJECT
TEST AREA NORTH/TECHNICAL SUPPORT FACILITY
PM-2A REMEDIATION PROJECT
TANK EXCAVATION PLAN

SHEET:	1 OF 1		SCALE:	SHOWN
DESIGN:	B.J. ORCHARD	03/17/04	QA LEVEL: Q	DWG SIZE: D
DRAWN:	B.D. WELTY	03/17/04	CONTRACT NO.	2073.00
CHECKED:	J.A. TOWERS	03/17/04	DRAWING NUMBER: P-FFA/CO-PM2A-001	REV 2
APPROVED:	B.J. ORCHARD	03/18/04		

Appendix C

Example UTEXAS3 Modeling Results and Example Output

Table C-1. PM-2A excavation slope stability modeling summary.

Trial/Engineer	Input Filename	General Notes	Slope (H:V)	Excavation Depth (ft)	Surcharge Loading	Material Properties	Failure Mode	Lowest Computed FOS
Trials 1-6: Comparison Between UTEXAS3 and SNAILZ Results								
1 – Intrepid	spols pile	Result for tank spoils loading or Case 1 from the Intrepid Engineering Report.	1:1	19.0	3,000 psf/ft imposed 10 to 17 ft from excavation slope crest, 1.5:1 spoils pile slope beginning 20 ft from excavation slope crest	See Note A below	Non-circular, toe failure	4.17
2 – Portage	si.txt	This result is from UTEXAS3 modeling for comparison to Trial 1. A lower factor of safety was found by using a search for circular failures.	1:1	19.0	3,000 psf/ft imposed 10 to 17 ft from excavation slope crest, 1.5:1 spoils pile slope beginning 20 ft from excavation slope crest	See Note A below	Circular, toe failure	3.63
3 – Intrepid	GMK5240 97kip SW exc	Result for crane surcharge loading or Case 2 from the Intrepid Engineering Report.	1:1	19.0	2,015 psf/ft imposed 20 to 28 ft from excavation slope crest	See Note A below	Non-circular, toe failure	5.21
4 – Portage	si2.txt	This result is from UTEXAS3 modeling for comparison to Trial 3. A lower factor of safety was found by using a search for circular failures.	1:1	19.0	2,015 psf/ft imposed 20 to 28 ft from excavation slope crest	See Note A below	Circular, toe failure	4.85
5 - Intrepid	GMK5240 97kip SW ramp	Result for crane surcharge loading or Case 3 from the Intrepid Engineering Report.	1:1	14.3	2,015 psf/ft imposed 9.4 to 17.4 ft from excavation slope crest	See Note A below	Non-circular, toe failure	4.68
6 – Portage	si3.txt	This result is from UTEXAS3 modeling for comparison to Trial 5. A lower factor of safety was found by using a search for circular failures.	1:1	14.3	2,015 psf/ft imposed 9.4 to 17.4 ft from excavation slope crest	See Note A below	Circular, toe failure	4.40
Trials 7-9: UTEXAS3 Results Using Phi = 0								
7 – Portage	sib.txt	Similar to Trial 2	1.1	19.0	3,000 psf/ft imposed 10 to 17 ft from excavation slope crest, 1.5:1 spoils pile slope beginning 20 ft from excavation slope crest	See Note B below	Circular, toe failure	2.84

Table C-1. (continued).

Trial/Engineer	Input Filename	General Notes	Slope (H:V)	Excavation Depth (ft)	Surcharge Loading	Material Properties	Failure Mode	Lowest Computed FOS
8 – Portage	si2b.txt	Similar to Trial 4	1:1	19.0	2,015 psf/ft imposed 20 to 28 ft from excavation slope crest	See Note B below	Circular, toe failure	3.77
9 – Portage	si3b.txt	Similar to Trial 6	1:1	14.3	2,015 psf/ft imposed 9.4 to 17.4 ft from excavation slope crest	See Note B below	Circular, toe failure	3.47
Trials 10-12: UTEXAS3 Results Using Phi = 0 and 1.5:1 Slope								
10 – Portage	sibs.txt	Similar to Trial 2	1.5:1	19.0	3,000 psf/ft imposed 10 to 17 ft from excavation slope crest, 1.5:1 spoils pile slope beginning 20 ft from excavation slope crest	See Note B below	Circular, toe failure	3.22
11 – Portage	si2bs.txt	Similar to Trial 4	1.5:1	19.0	2,015 psf/ft imposed 20 to 28 ft from excavation slope crest	See Note B below	Circular, toe failure	4.62
12 – Portage	si3bs.txt	Similar to Trial 6	1.5:1	14.3	2,015 psf/ft imposed 9.4 to 17.4 ft from excavation slope crest	See Note B below	Circular, toe failure	4.79
Trials 13-19: UTEXAS3 Results for Currently Proposed (March, 2004) Excavation Design Slopes								
13 – Portage	scnb.txt	North slope, next to Snake Avenue	1:1	17.0	No surcharge	See Note B below	Circular, toe failure	5.60
14 - Portage	scnb2.txt	North slope, next to Snake Avenue	1:1	17.0	3,000 psf/ft imposed 10 to 17 ft from excavation slope crest	See Note B below	Circular, toe failure	2.71
15 - Portage	scnc.txt	North slope, next to Snake Avenue. Soil strength parameters greatly reduced to assess the result.	1:1	17.0	No surcharge	See Note C below	Circular, toe failure	1.26
16 - Portage	scnc2.txt	North slope, next to Snake Avenue. Soil strength parameters greatly reduced to assess the result.	1:1	17.0	3,000 psf/ft imposed 10 to 17 ft from excavation slope crest	See Note C below	Circular, toe failure	0.84
17 - Portage	sceb.txt	East slope, crane pad side. Crane track parallel to excavation slope crest.	1.5:1	17.0	4,200 psf/ft applied 10-14 ft and 36-40 ft from exc. slope crest	See Note B below	Circular, toe failure	1.75
18 - Portage	sceb2.txt	East slope, crane pad side. Crane track parallel to excavation slope crest.	1.5:1	17.0	4,200 psf/ft applied 20-24 ft and 46-50 ft from exc. slope crest	See Note B below	Circular, toe failure	2.47
19 - Portage	sceb3.txt	East slope, crane pad side. Crane track perpendicular to excavation slope crest.	1.5:1	17.0	1,300 psf/ft applied 10 - 36.75 ft from exc. slope crest	See Note B below	Circular, toe failure	3.65

NOTES:
A. Intrepid's material properties used for modeling: cohesion, c = 2,000 psf; angle of internal friction, phi = 25 degrees; total unit weight, gamma = 110 pcf.
B. No internal angle of friction; material properties used for modeling: cohesion, c = 2,000 psf; angle of internal friction, phi = 0 degrees; total unit weight, gamma = 110 pcf.
C. Greatly reduced soil strength parameters; material properties used for modeling: cohesion, c = 250 psf; angle of internal friction, phi = 15 degrees; total unit weight, gamma = 110 pcf.

```
UTEXAS3 - VER. 1.209 -  2/28/98 - (C) 1985-2000 S. G. WRIGHT
          One copy licensed to Portage, Engineering Division, Helena, MT
          Date:  3:29:2004   Time: 11:45: 4   Input file: a:sceb3.txt


          TABLE NO. 1
          ******************************************
          * COMPUTER PROGRAM DESIGNATION - UTEXAS3 *
          * Originally Coded By Stephen G. Wright  *
          * Version No. 1.209                      *
          * Last Revision Date  2/28/98            *
          * (C) Copyright 1985-1998 S. G. Wright   *
          * All Rights Reserved                    *
          ******************************************

          **************************************************************
          *                                                            *
          *      RESULTS OF COMPUTATIONS PERFORMED USING THIS COMPUTER *
          * PROGRAM SHOULD NOT BE USED FOR DESIGN PURPOSES UNLESS THEY *
          * HAVE BEEN VERIFIED BY INDEPENDENT ANALYSES, EXPERIMENTAL   *
          * DATA OR FIELD EXPERIENCE. THE USER SHOULD UNDERSTAND THE   *
          * ALGORITHMS AND ANALYTICAL PROCEDURES USED IN THE COMPUTER  *
          * PROGRAM AND MUST HAVE READ ALL DOCUMENTATION FOR THIS      *
          * PROGRAM BEFORE ATTEMPTING ITS USE.                         *
          *                                                            *
          *      NEITHER SHINOAK SOFTWARE NOR STEPHEN G. WRIGHT        *
          * MAKE OR ASSUME LIABILITY FOR ANY WARRANTIES, EXPRESSED OR  *
          * IMPLIED, CONCERNING THE ACCURACY, RELIABILITY, USEFULNESS  *
          * OR ADAPTABILITY OF THIS COMPUTER PROGRAM.                  *
          *                                                            *
          **************************************************************
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               PM2A Tank Excavation, Proposed East Slope With Crane
               Static Condition g=0, Toe Failure


          TABLE NO.  2
          *************************
          * NEW PROFILE LINE DATA *
          *************************

          PROFILE LINE  1 - MATERIAL TYPE =  1
          profile with uniform soils

               Point      X             Y

                 1          .000          .000
                 2        20.000          .000
                 3        45.500        17.000
                 4        95.500        17.000

          All new profile lines defined - No old lines retained
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               Static Condition g=0, Toe Failure


          TABLE NO.  3
          ***********************************************************************
```

```
* NEW MATERIAL PROPERTY DATA - CONVENTIONAL/FIRST-STAGE COMPUTATIONS *
**********************************************************************

DATA FOR MATERIAL TYPE  1
embankment soils

     Unit weight of material =  110.000

     CONVENTIONAL (ISOTROPIC) SHEAR STRENGTHS
     Cohesion - - - - - - - -    2000.000
     Friction angle - - - - -    .000 degrees

     No (or zero) pore water pressures

All new material properties defined - No old data retained
```

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```
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     Static Condition g=0, Toe Failure


TABLE NO. 10
*********************************************************************
* NEW SURFACE PRESSURE DATA - CONVENTIONAL/FIRST-STAGE COMPUTATIONS *
*********************************************************************

ALL NEW DATA INPUT - NO OLD DATA RETAINED

Surface Pressures -
```

Point	X	Y	Normal Pressure	Shear Stress
1	55.500	17.000	1300.000	.000
2	82.250	17.000	1300.000	.000

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```
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     Static Condition g=0, Toe Failure


TABLE NO. 15
*********************************
* NEW ANALYSIS/COMPUTATION DATA *
*********************************

Circular Shear Surface(s)

Automatic Search Performed

Starting Center Coordinate for Search at -
                                             X =      25.000
                                             Y =      30.000

Required accuracy for critical center (= minimum
spacing between grid points) =       1.000

Critical shear surface not allowed to pass below Y =      -7.500

For the initial mode of search
all circles pass through the point at -
                                             X =      20.000
```

Y = .000

Depth of crack = .000

Initial trial estimate for the factor of safety = 3.000

Short form of output will be used for search

THE FOLLOWING REPRESENT EITHER DEFAULT OR PREVIOUSLY DEFINED VALUES:

Initial trial estimate for side force inclination = 15.000 degrees
(Applicable to Spencer's procedure only)

Maximum number of iterations allowed for
calculating the factor of safety = 40

Allowed force imbalance for convergence = 100.000

Allowed moment imbalance for convergence = 100.000

Initial trial values for factor of safety (and side force inclination
for Spencer's procedure) will be kept constant during search

Maximum subtended angle to be used for subdivision of the
circle into slices = 3.00 degrees

Search will be continued to locate a more critical shear
surface (if one exists) after the initial mode is complete

Depth of water in crack = .000

Unit weight of water in crack = 62.400

Seismic coefficient = .000

Conventional (single-stage) computations to be performed

Procedure used to compute the factor of safety: SPENCER

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PM2A Tank Excavation, Proposed East Slope With Crane
Static Condition g=0, Toe Failure

TABLE NO. 16
****
* NEW SLOPE GEOMETRY DATA *
****

NOTE - NO DATA WERE INPUT, SLOPE GEOMETRY DATA
WERE GENERATED BY THE PROGRAM

Slope Coordinates -

Point	X	Y
1	.000	.000
2	20.000	.000
3	45.500	17.000
4	95.500	17.000

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PM2A Tank Excavation, Proposed East Slope With Crane
Static Condition g=0, Toe Failure

TABLE NO. 20

```
*********************************************************
* SHORT-FORM TABLE FOR SEARCH WITH CIRCULAR SHEAR SURFACES *
*********************************************************
```

Mode	Center Coordinates of Critical Circle X	Center Coordinates of Critical Circle Y	Radius	1-Stage Factor of Safety	Side Force Inclin.
1 Fixed Point at X = 20.0 Y = .0	43.000	37.000	43.566	3.651	11.34
2 Tangent Line at Y = -6.6	42.000	39.000	45.566	3.646	11.41
3 Constant Radius of R = 45.6	42.000	39.000	45.566	3.646	11.41

TABLE NO. 21

```
*****  1-STAGE FINAL CRITICAL CIRCLE INFORMATION  *****
X Coordinate of Center - - - - - - - -    42.000
Y Coordinate of Center - - - - - - - -    39.000
Radius - - - - - - - - - - - - - - - -    45.566
Factor of Safety - - - - - - - - - - -     3.646
Side Force Inclination - - - - - - - -     11.41

Number of circles tried  - - - - - -  156
No. of circles F calc. for - - - - -  117


***** CAUTION ***** FACTOR OF SAFETY COULD NOT BE COMPUTED FOR SOME
                    OF GRID POINTS AROUND THE MINIMUM
***** RESULTS MAY BE ERRONEOUS *****
```

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PM2A Tank Excavation, Proposed East Slope With Crane
Static Condition g=0, Toe Failure

TABLE NO. 26

```
**************************************************************
*  Coordinate, Weight, Strength and Pore Water Pressure      *
*  Information for Individual Slices for Conventional        *
*  Computations or First Stage of Multi-Stage Computations.  *
*  (Information is for the Critical Shear Surface in the     *
*  Case of an Automatic Search.)                             *
**************************************************************
```

Slice No.	X	Y	Slice Weight	Matl. Type	Cohesion	Friction Angle	Pore Pressure
	18.4	.0					
1	19.2	-.5	77.7	1	2000.00	.00	.0
	20.0	-.9					
2	21.1	-1.5	502.8	1	2000.00	.00	.0

	22.1	-2.0					
3	23.2	-2.5	1106.4	1	2000.00	.00	.0
	24.3	-3.0					
4	25.4	-3.4	1715.6	1	2000.00	.00	.0
	26.5	-3.9					
5	27.6	-4.2	2322.6	1	2000.00	.00	.0
	28.8	-4.6					
6	29.9	-4.9	2919.8	1	2000.00	.00	.0
	31.1	-5.2					
7	32.2	-5.5	3500.0	1	2000.00	.00	.0
	33.4	-5.7					
8	34.6	-5.9	4056.0	1	2000.00	.00	.0
	35.8	-6.1					
9	36.9	-6.3	4581.3	1	2000.00	.00	.0
	38.1	-6.4					
10	39.3	-6.5	5069.8	1	2000.00	.00	.0
	40.5	-6.5					
11	41.3	-6.6	3388.8	1	2000.00	.00	.0
	42.0	-6.6					
12	43.2	-6.5	5770.1	1	2000.00	.00	.0
	44.4	-6.5					
13	44.9	-6.5	2833.3	1	2000.00	.00	.0
	45.5	-6.4					
14	46.7	-6.3	6084.0	1	2000.00	.00	.0
	47.9	-6.2					
15	49.1	-6.0	5963.1	1	2000.00	.00	.0
	50.2	-5.8					
16	51.4	-5.6	5795.4	1	2000.00	.00	.0
	52.6	-5.3					
17	53.7	-5.0	5583.3	1	2000.00	.00	.0
	54.9	-4.7					
18	55.2	-4.6	1499.8	1	2000.00	.00	.0
	55.5	-4.5					
19	56.6	-4.1	5252.8	1	2000.00	.00	.0
	57.8	-3.8					
20	58.9	-3.3	4951.3	1	2000.00	.00	.0
	60.0	-2.9					

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PM2A Tank Excavation, Proposed East Slope With Crane
Static Condition g=0, Toe Failure

TABLE NO. 26

```
*************************************************************
*  Coordinate, Weight, Strength and Pore Water Pressure      *
*  Information for Individual Slices for Conventional        *
*  Computations or First Stage of Multi-Stage Computations.  *
*  (Information is for the Critical Shear Surface in the     *
*  Case of an Automatic Search.)                             *
*************************************************************
```

Slice No.	X	Y	Slice Weight	Matl. Type	Cohesion	Friction Angle	Pore Pressure
	60.0	-2.9					
21	61.1	-2.4	4617.0	1	2000.00	.00	.0
	62.1	-1.9					
22	63.2	-1.3	4254.5	1	2000.00	.00	.0
	64.3	-.8					
23	65.3	-.2	3868.9	1	2000.00	.00	.0
	66.3	.5					

24	67.3	1.1	3465.7	1	2000.00	.00	.0
	68.3	1.8					
25	69.2	2.5	3050.5	1	2000.00	.00	.0
	70.2	3.2					
26	71.1	4.0	2629.3	1	2000.00	.00	.0
	72.0	4.7					
27	72.9	5.5	2208.4	1	2000.00	.00	.0
	73.8	6.4					
28	74.6	7.2	1793.8	1	2000.00	.00	.0
	75.5	8.1					
29	76.2	9.0	1392.1	1	2000.00	.00	.0
	77.0	9.9					
30	77.8	10.8	1009.2	1	2000.00	.00	.0
	78.5	11.7					
31	79.2	12.7	651.4	1	2000.00	.00	.0
	79.9	13.7					
32	80.5	14.7	324.6	1	2000.00	.00	.0
	81.2	15.7					
33	81.5	16.3	54.0	1	2000.00	.00	.0
	81.9	17.0					

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Static Condition g=0, Toe Failure

TABLE NO. 27

```
************************************************************
*  Seismic Forces and Forces Due to Surface Pressures for   *
*  Individual Slices for Conventional  Computations or the  *
*  First Stage of Multi-Stage Computations.                 *
*  (Information is for the Critical Shear Surface in the    *
*  Case of an Automatic Search.)                            *
************************************************************
```

				FORCES DUE TO SURFACE PRESSURES			
Slice No.	X	Seismic Force	Y for Seismic Force	Normal Force	Shear Force	X	Y
1	19.2	0.	-.2	0.	0.	19.2	.0
2	21.1	0.	-.4	0.	0.	21.1	.7
3	23.2	0.	-.2	0.	0.	23.2	2.1
4	25.4	0.	.1	0.	0.	25.4	3.6
5	27.6	0.	.4	0.	0.	27.6	5.1
6	29.9	0.	.8	0.	0.	29.9	6.6
7	32.2	0.	1.3	0.	0.	32.2	8.2
8	34.6	0.	1.9	0.	0.	34.6	9.7
9	36.9	0.	2.5	0.	0.	36.9	11.3
10	39.3	0.	3.2	0.	0.	39.3	12.9
11	41.3	0.	3.8	0.	0.	41.3	14.2
12	43.2	0.	4.5	0.	0.	43.2	15.5
13	44.9	0.	5.1	0.	0.	44.9	16.6
14	46.7	0.	5.3	0.	0.	46.7	17.0
15	49.1	0.	5.5	0.	0.	49.1	17.0
16	51.4	0.	5.7	0.	0.	51.4	17.0
17	53.7	0.	6.0	0.	0.	53.7	17.0
18	55.2	0.	6.2	0.	0.	55.2	17.0
19	56.6	0.	6.4	2937.	0.	56.6	17.0
20	58.9	0.	6.8	2881.	0.	58.9	17.0
21	61.1	0.	7.3	2817.	0.	61.1	17.0
22	63.2	0.	7.8	2745.	0.	63.2	17.0

```
   23     65.3        0.         8.4    2666.       0.     65.3     17.0
   24     67.3        0.         9.1    2579.       0.     67.3     17.0
   25     69.2        0.         9.7    2485.       0.     69.2     17.0
   26     71.1        0.        10.5    2385.       0.     71.1     17.0
   27     72.9        0.        11.3    2278.       0.     72.9     17.0
   28     74.6        0.        12.1    2165.       0.     74.6     17.0
   29     76.2        0.        13.0    2045.       0.     76.2     17.0
   30     77.8        0.        13.9    1921.       0.     77.8     17.0
   31     79.2        0.        14.9    1791.       0.     79.2     17.0
   32     80.5        0.        15.8    1656.       0.     80.5     17.0
   33     81.5        0.        16.7     975.       0.     81.5     17.0
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          TABLE NO. 29
          ********************************************************************
          *  Information Generated During Iterative Solution for the Factor  *
          *  of Safety and Side Force Inclination by Spencer's Procedure     *
          ********************************************************************

                  Trial     Trial
                 Factor   Side Force    Force      Moment                Delta
          Iter-    of    Inclination  Imbalance  Imbalance   Delta-F     Theta
          ation  Safety   (degrees)     (lbs.)   (ft.-lbs.)            (degrees)

             1   3.00000    15.0000  -.1126E+05  -.9270E+05
          First-order corrections to F and THETA .........   .414E+00 -.874E+01
          Values factored by  .983E+00 - Deltas too large    .407E+00 -.859E+01

             2   3.40684     6.4056  -.1169E+04  -.8862E+05
          First-order corrections to F and THETA .........   .232E+00  540E+01
          Second-order correction - Iteration   1 ........   .253E+00  540E+01
          Second-order correction - Iteration   2 ........   .254E+00  540E+01

             3   3.66051    11.8013   .2738E+02   .6711E+04
          First-order corrections to F and THETA .........  -.151E-01 -.388E+00
          Second-order correction - Iteration   1 ........  -.150E-01 -.388E+00
          Second-order correction - Iteration   2 ........  -.150E-01 -.388E+00

             4   3.64554    11.4137  -.5386E-02   .1949E+02
          First-order corrections to F and THETA .........  -.384E-04 -.118E-02

          Factor of Safety - - - - - - - -     3.646
          Side Force Inclination - - - - -     11.41
          Number of Iterations - - - - - -     4
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               Static Condition g=0, Toe Failure


          TABLE NO. 38
          ***************************************************************
          *  Final Results for Stresses Along the Shear Surface         *
          *  (Results for Critical Shear Surface in Case of a Search.)  *
          ***************************************************************

          SPENCER'S PROCEDURE USED TO COMPUTE FACTOR OF SAFETY
```

Factor of Safety = 3.646   Side Force Inclination = 11.41 Degrees

-------- VALUES AT CENTER OF BASE OF SLICE---------

Slice No.	X-center	Y-center	Total Normal Stress	Effective Normal Stress	Shear Stress
1	19.2	-.5	540.2	540.2	548.6
2	21.1	-1.5	705.9	705.9	548.6
3	23.2	-2.5	955.8	955.8	548.6
4	25.4	-3.4	1191.8	1191.8	548.6
5	27.6	-4.2	1413.6	1413.6	548.6
6	29.9	-4.9	1620.8	1620.8	548.6
7	32.2	-5.5	1813.4	1813.4	548.6
8	34.6	-5.9	1990.8	1990.8	548.6
9	36.9	-6.3	2152.9	2152.9	548.6
10	39.3	-6.5	2299.3	2299.3	548.6
11	41.3	-6.6	2407.4	2407.4	548.6
12	43.2	-6.5	2502.8	2502.8	548.6
13	44.9	-6.5	2582.1	2582.1	548.6
14	46.7	-6.3	2564.4	2564.4	548.6
15	49.1	-6.0	2476.6	2476.6	548.6
16	51.4	-5.6	2376.7	2376.7	548.6
17	53.7	-5.0	2265.0	2265.0	548.6
18	55.2	-4.6	2189.0	2189.0	548.6
19	56.6	-4.1	3322.3	3322.3	548.6
20	58.9	-3.3	3171.2	3171.2	548.6
21	61.1	-2.4	3009.1	3009.1	548.6
22	63.2	-1.3	2836.2	2836.2	548.6
23	65.3	-.2	2653.0	2653.0	548.6
24	67.3	1.1	2459.7	2459.7	548.6
25	69.2	2.5	2256.9	2256.9	548.6
26	71.1	4.0	2044.8	2044.8	548.6
27	72.9	5.5	1824.0	1824.0	548.6
28	74.6	7.2	1594.9	1594.9	548.6
29	76.2	9.0	1357.9	1357.9	548.6
30	77.8	10.8	1113.4	1113.4	548.6
31	79.2	12.7	861.9	861.9	548.6
32	80.5	14.7	603.7	603.7	548.6
33	81.5	16.3	388.6	388.6	548.6

CHECK SUMS - (ALL SHOULD BE SMALL)
SUM OF FORCES IN VERTICAL DIRECTION = .00 (= 384E-02)
SHOULD NOT EXCEED .100E+03
SUM OF FORCES IN HORIZONTAL DIRECTION = .01 (= 101E-01)
SHOULD NOT EXCEED .100E+03
SUM OF MOMENTS ABOUT COORDINATE ORIGIN = -19.57 (= -.196E+02)
SHOULD NOT EXCEED .100E+03
SHEAR STRENGTH/SHEAR FORCE CHECK-SUM = .00 (= 153E-02)
SHOULD NOT EXCEED .100E+03

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Static Condition g=0, Toe Failure

TABLE NO. 39
****
* Final Results for Side Forces and Stresses Between Slices. *
* (Results for Critical Shear Surface in Case of a Search.) *

****

SPENCER'S PROCEDURE USED TO COMPUTE FACTOR OF SAFETY
Factor of Safety = 3.646 Side Force Inclination = 11.41 Degrees

--------------- VALUES AT RIGHT SIDE OF SLICE ----------------

Slice No.	X-Right	Side Force	Y-Coord. of Side Force Location	Fraction of Height	Sigma at Top	Sigma at Bottom
1	20.0	1373.	-.3	.675	3053.2	-72.7
2	22.1	3349.	-.7	.388	316.7	1607.1
3	24.3	5525.	-1.0	.333	-2.6	1855.5
4	26.5	7825.	-1.4	.305	-160.4	2032.2
5	28.8	10176.	-1.6	.286	-271.3	2179.2
6	31.1	12509.	-1.8	.271	-360.6	2303.4
7	33.4	14759.	-1.9	.259	-436.5	2406.5
8	35.8	16865.	-2.0	.249	-502.5	2488.9
9	38.1	18773.	-2.0	.240	-560.1	2550.6
10	40.5	20435.	-1.9	.231	-610.0	2591.5
11	42.0	21326.	-1.8	.225	-637.2	2606.3
12	44.4	22501.	-1.6	.217	-674.8	2613.0
13	45.5	22936.	-1.4	.214	-689.8	2608.7
14	47.9	23621.	-1.1	.219	-682.4	2679.7
15	50.2	24008.	-.7	.224	-676.4	2739.1
16	52.6	24121.	-.3	.227	-675.9	2794.1
17	54.9	23993.	.2	.228	-686.1	2852.6
18	55.5	23920.	.4	.228	-691.5	2870.6
19	57.8	22588.	1.1	.234	-632.7	2766.4
20	60.0	20970.	1.9	.241	-573.0	2641.9
21	62.1	19119.	2.8	.247	-513.3	2499.3
22	64.3	17089.	3.7	.253	-454.2	2340.3
23	66.3	14937.	4.7	.259	-396.5	2166.5
24	68.3	12723.	5.8	.264	-340.4	1979.3
25	70.2	10508.	6.9	.269	-286.3	1780.0
26	72.0	8353.	8.1	.275	-234.5	1569.5
27	73.8	6322.	9.3	.280	-184.8	1348.7
28	75.5	4474.	10.6	.287	-137.2	1118.3
29	77.0	2871.	12.0	.295	-90.9	878.4
30	78.5	1570.	13.4	.308	-43.8	627.5
31	79.9	628.	14.8	.343	11.0	359.5
32	81.2	99.	16.5	.595	116.7	31.7
33	81.9	0.	3769.7	ABOVE	.0	.0

```
CHECK SUMS - (ALL SHOULD BE SMALL)
SUM OF FORCES IN VERTICAL DIRECTION    =         .00  (=  384E-02)
     SHOULD NOT EXCEED     .100E+03
SUM OF FORCES IN HORIZONTAL DIRECTION  =         .01  (=  101E-01)
     SHOULD NOT EXCEED     .100E+03
SUM OF MOMENTS ABOUT COORDINATE ORIGIN =      -19.57  (= -.196E+02)
     SHOULD NOT EXCEED     .100E+03
SHEAR STRENGTH/SHEAR FORCE CHECK-SUM   =         .00  (=  153E-02)
     SHOULD NOT EXCEED     .100E+03
```

END-OF-FILE ENCOUNTERED WHILE READING COMMAND
WORDS - END OF PROBLEM(S) ASSUMED

Appendix D
Bearing Capacity Calculations

Theory (refer to Das, 1984)

Based on Terzaghi's theory and methods, the ultimate bearing capacity for a shallow strip foundation can be given by:

$q_u = cN_c + qN_q + ½ \gamma BN_\gamma$

where q_u = ultimate bearing pressure

c = cohesion of soil

γ = unit weight of soil

$q = \gamma D_f$, where D_f is the footing embedment depth below grade

B = footing width

and N_c, N_q, and N_γ are nondimensional bearing capacity factors that are functions of the soil friction angle, Φ.

Similarly, for foundations that exhibit the local shear failure mode in soils, the ultimate bearing capacity for a shallow strip foundation can be given by:

$q_u = (2/3)cN_c' + qN_q' + ½ \gamma BN_\gamma'$

where modified bearing capacity factors are used.

Given

c = 4,000 psf

γ = 110 pcf

q = 0 (the track is at ground surface; there is no embedment depth)

B = 4.0 feet

Φ = 0 degrees, no internal angle of friction is assumed for the clay soil

For the case of $\Phi = 0$: N_c,= N_c' = 5.7; $N_q = N_q'$ = 1; and $N_\gamma = N_\gamma'$ = 0.

Results

For the case of $\Phi = 0$, the second two terms in both equations become zero and the local shear failure case yields the controlling result, as follows:

$q_u = (2/3)cN_c'$ = (2/3)(4,000 psf)(5.7) = 15,2000 psf

Applying the standard factor of safety (FOS) of 3.0 for bearing capacity analysis gives an allowable bearing pressure of:

$q_{all} = q_u$ / 3.0 = 15,200 psf / 3.0 = 5,067 psf. >> Use 5,000 psf.